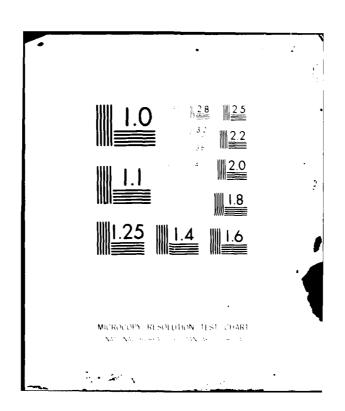
NEW YORK STATE DEPT OF ENVIRONMENTAL CONSERVATION ALBANY F/G 13/13 NATIONAL DAM SAFETY PROGRAM. OTISCO LAKE DAM (INVENTORY NUMBER --ETC(U) SEP 81 G KOCH AD-A109 965 UNCLASSIFIED NL L ... 2



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inspection of the dam by the performing organization.

The examination of documents and the visual inspection of Otisco Lake Dam did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

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Further investigations are required to assess the stability of the spillway section. Analysis performed indicated that the structure is only marginally stable under normal loading conditions and is unstable when subjected to severe loading conditions (such as flood flows or ice loading).

The spillway has sufficient capacity to discharge the Probable Maximum Flood (PMF) therefore, it has been assessed as "Adequate" according to the Corps of Engineers screening criteria.

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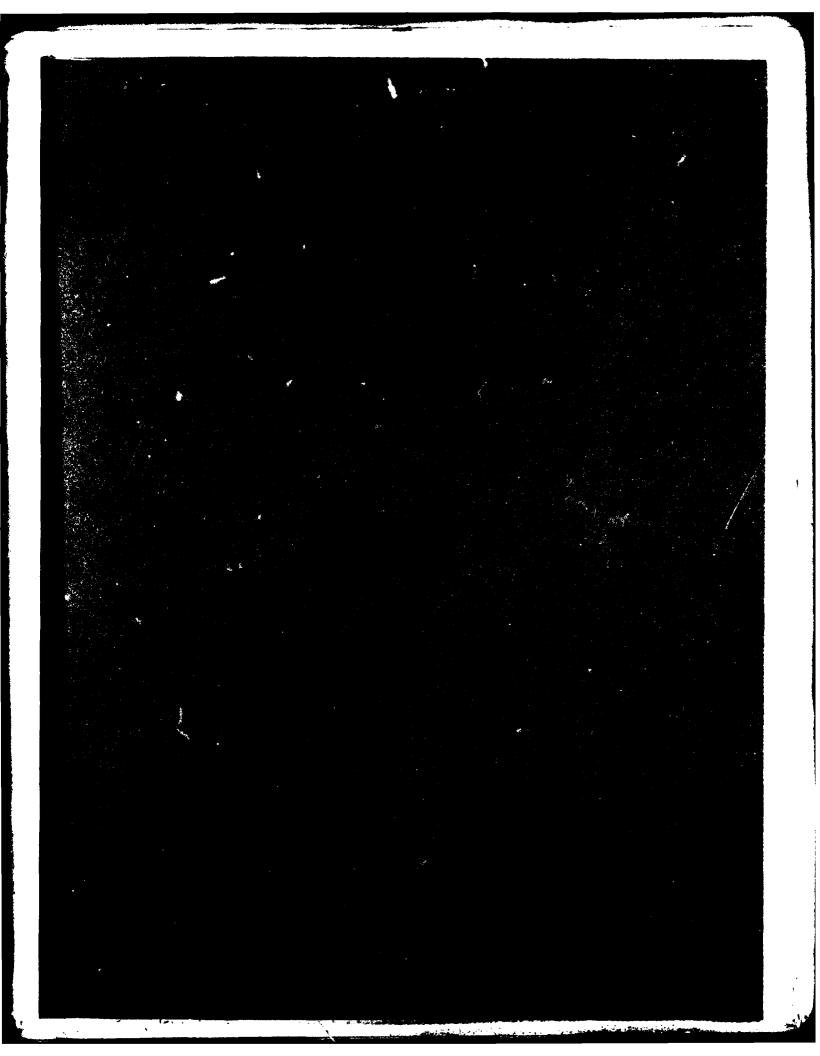
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PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.



PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM OTISCO LAKE DAM I.D. NO. NY-753 DEC #73B-2751A OSWEGO RIVER BASIN ONONDAGA COUNTY

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PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM

Name of Dam:

Otisco Lake Dam I.D. No. NY 753

State Located:

New York

County:

Onandaga

Watershed:

Oswego River Basin

Stream:

Ninemile Creek:

tributary to Onandaga Lake

Date of Inspection:

June 12, 1981

ASSESSMENT:

The examination of documents and the visual inspection of Otisco Lake Dam did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

Further investigations are required to assess the stability of the spillway section. Analysis performed indicated that the structure is only marginally stable under normal loading conditions and is unstable when subjected to severe loading conditions, (such as flood flows or ice loading).

The spillway has sufficient capacity to discharge the Probable Maximum Flood (PMF) therefore, it has been assessed as "Adequate" according to the Corps of Engineers screening criteria.

In addition, the dam has several problem areas which if left uncorrected, have the potential for the development of hazardous conditions and must be corrected within 1 year. These areas are:

- Repair the deteriorated portions of the concrete on the downstream edges of the concrete paving.
- 2. Repair the undermining of the concrete paving.
 - Provide a program of periodic inspection and maintenance of the dam and appurtenances, Document this information for future reference.
 - 4. An emergency action plan must be developed and maintained during the life of the structure.

George Koch

Chief, Dam Safety Section
New York State Department
of Environmental Conservation
NY License No. 45937

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Filliand G. Mayo JC upo Col. W. M. Smith, Jr. New York District Engineer

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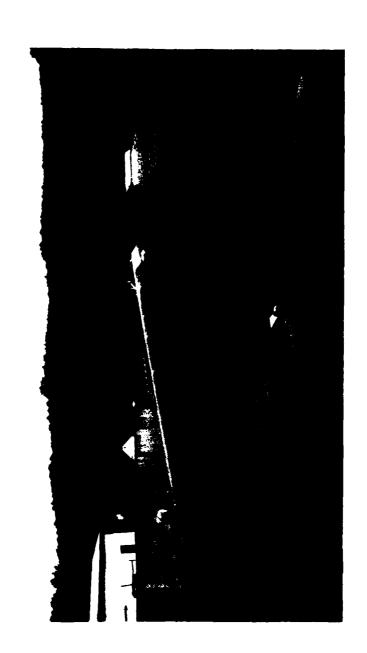
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OTISCO LAKE DAM OVERVIEW

PHASE I INSPECTION REPORT NATIONAL DAM SAFETY PROGRAM OTISCO LAKE DAM I.D. NO. NY753 DEC # 73B-2751A OSWEGO RIVER BASIN ONONDAGA COUNTY

SECTION 1: PROJECT INFORMATION

1.1 GENERAL

a. Authority
The Phase I inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers, to fulfill the requirements of the National Dam Inspection Act, Public Law 92-367.

b. Purpose of Inspection
This inspection was conducted to evaluate the existing conditions of the dam, to identify deficiencies and hazardous conditions, to determine if these deficiencies constitute hazards to life and property, and to recommend remedial measures where required.

1.2 DESCRIPTION OF PROJECT

a. Description of Dam and Appurtenances
The Otisco Lake Dam is a 300 feet long earth dam which has an ogee
masonry/concrete primary spillway and a concrete paved secondary spillway abutted by reinforced concrete training walls. The masonry/concrete
spillway is 18 feet high and has 55 feet of ogee weir length. The
secondary spillway has a total length of 336 feet of uncontrolled overflow section. The secondary spillway channel within the training walls
is directed into the natural channel by the training walls. There is a
concrete core wall located under the overflow section and around the
abutment walls. There are three 4' x 4' low level outlets located
adjacent to the primary spillway section.

The dam is located on Ninemile Creek, which is tributary to Onondaga Lake, Seneca River and finally the Oswego River. The village of Marietta, New York is downstream of the dam, within one mile.

C. Size The dam is 18 feet high and impounds 26,000 acre feet at normal pool elevation. The dam is classified as "intermediate" in size.

d. Hazard Classification
The dam is classified as high hazard due to its location, above several low lying homes in the area between the dam and the village of Marietta, New York.

e. Ownership
The dam is owned by Onondaga County Water Authority, P.O. Box 9, Northern

Concourse, Syracuse, New York 13211. The person responsible for operation and maintenance, who was contacted to make the inspection of the dam was Mr. Joseph DeVoldre, (315) 455-7061.

f. Purpose of Dam The dam was built to increase storage for water supply.

Design and Construction History In 1857, a feeder dam for the Erie Canal was constructed at the site of the present dam. By 1872 the dam had been raised another ten feet for more storage for canal use. In 1907, a new dam of masonry construction (which now makes up the lower portion of the primary spillway) was completed adding another four feet of storage and providing a domestic water source. The dam as it now exists was completed in 1962.

Normal Operating Conditions All flows in excess of the Onondaga County Water Authority requirements are passed over the uncontrolled spillway. The three low level outlets are operable and in good working condition.

1.3 PERTINENT DATA

a. Drainage Area (sq.mi.)			
b. Elevations (ft., USGS datum) Top of Dam Secondary Spillway Primary Spillway Low Level Outlets Original Stream Channel		798.1 790.1 786.6 768.6 768.	±

c. Reservoir	
Surface Area @ spillway crest (acres)	2291.
Storage @ Top of dam (acre-feet)	47,890.
Storage @ Spillway Crest (acre-feet)	26,067.

Type: Earth fill with reinforced concrete paving on the downstream slope and concrete core wall, an earth embankment makes up the right portion of the dam.

Length (ft) Concrete paved overflow section	336.
Earth embankment	300.
Height (ft)	
Upstream Slope, Earth embankment	3H:1V
Downstream Slope, Earth Embankment	3H:1V
Crest Width, Earth Embankment	10.

e. Spillway
Type: Masonry/concrete ogee section

Weir length (ft)

55.

Spillway Capacity (cfs)
@ Secondary Spillway Crest
@ Top of Dam

1,369. 31,345.

f. Reservoir Drain
Type: Three - 4' x 4' sluiceways with manually controlled gate.

Capacity @ Normal Pool Elevation (cfs)

750.

SECTION 2: ENGINEERING DATA

2.1 GEOTECHNICAL DATA

a. Geology
The Otisco Lake Dam is located in the "Alleghany Plateau" physiographic province of New York State. The hills are smoothly sloping and generally well drained. The deep soils of the area are of the Honeoye-Lima Association.

b. Subsurface Investigation
No information about foundation conditions or borings could be found other than some description from the 1958 Dam Reconstruction Application. This stated the character of the stream bed and banks as "silty sand with traces of gravel, compact sand and gravel, and silty clay in various parts".

c. Design Records
There are no design records for the original dam constructed at the site. The only records available on the most recent reconstruction are plans which are included in App.F. Drawings or can be obtained from 0'Brien and Gere, Engineers at 1304 Buckley Road, Syracuse, N.Y. (315) 451-4700.

2.2 CONSTRUCTION RECORDS

There are no construction records available for the original construction or reconstruction of Otisco Lake Dam.

2.3 OPERATION RECORDS

Operating records are available at the water station located adjacent to the dam. Any other data available regarding water levels, usage, or quality can be obtained from Mr. Joseph DeVoldre, Water Plant Manager, (315) 455-7061.

2.4 EVALUATION OF DATA

The data presented in this report is compiled from information contained in the files of Department of Environmental Conservation, drawings from O'Brien and Gere, Engineers, and data gained from the visual inspection. This information appears to be adequate and reliable for Phase I Inspection purposes.

SECTION 3: VISUAL INSPECTION

3:1 FINDINGS

a. General
Visual inspection of the Otisco Lake Dam and surrounding watershed
was conducted on June 12, 1981. The weather was cloudy and the
temperature ranged in the seventies. The reservoir water surface
was at the primary spillway crest.

Both the earth embankments and the concrete paved section of the embankment appear to be in very good condition and well maintained. The vertical and horizontal alignments of the embankments are good. Some minor deterioration of concrete and minor undermining were found at the downstream edge of the secondary spillway (Photos 6 & 7). There is a roadway that passes between the spillway training wall and the right earth embankment. (See Photo #5). The road elevation is approximately 2.5 feet above the secondary spillway crest, therefore, under extreme flooding conditions, it would be possible for flow to pass over the dam on the roadway before overtopping occurs. There are stop logs available at the dam to block off the road, preventing erosion and hazard due to extreme floods. However, there is no emergency action plan detailing how or when to implement a plan.

c. Seepage There was no seepage or sloughing found at the toe or any of the slopes.

d. Spillway
The overflow spillway is in good condition. Both the masonry and concrete are well maintained. The secondary spillway concrete paving is also in good condition.

e. Reservoir Drain
There are three 4'x 4' sluiceways located adjacent to the primary spillway. The manually operated valves are in good working condition, located at the secondary spillway elevation. A 36 inch and 24 inch pipe are also available to draw off water into the supply system.

f. Reservoir
Originally a naturally occurring lake, the area around it appears very stable. Sedimentation is not a problem with the dam at present.

3.2 EVALUATION OF OBSERVATIONS

The only deficiencies that could be found with the visual inspection was the small amount of concrete deterioration and slight undermining of the concrete paving on the left side of the spillway channel.

SECTION 4: OPERATION AND MAINTENANCE PROCEDURES

4.1 PROCEDURES

The normal water surface is approximated by the uncontrolled overflow primary spillway. The other draw from the reservoir is through a 36 inch or 24 inch pipeline to the Onondaga County Water Authority distribution system.

4.2 MAINTENANCE OF THE DAM

The dam is maintained by the Onondaga County Water Authority. Maintenance is considered satisfactory. All valves and gates are operated and lubricated annually.

4.3 WARNING SYSTEM

There is no warning system in effect.

4.4 EVALUATION

The dam and appurtenances have been maintained in a satisfactory condition, except for the minor maintenance noted in this report.

SECTION 5: HYDROLOGIC/HYDRAULIC

5.1 DRAINAGE AREA CHARACTERISTICS

Otisco Lake Dam is located about 3/4 mile southeast of the Village of Marietta. The lake is fed at its upper end by Spafford Creek, while downstream of the dam, the excess flow is discharged into Ninemile Creek. The total drainage area of the basin is 39.15 square miles and the surface area of the lake at normal pool is 3.71 square miles. The basin drains generally in a northerly direction. Its slope ranges from moderate to steep and, except for some marshy areas south of the lake, is fairly well drained. It was analysed as a single basin.

5.2 ANALYSIS CRITERIA

The analysis of the spillway capacity of the dam and storage of the reservoir was performed using the Corps of Engineers HEC-1 computer program incorporating the "Snyder Synthetic Unit Hydrograph" method and the "Modified Puls" flood routing procedure. The floods selected for analysis were the PMF and 1/2 the PMF in accordance with the recommended guidelines of the Corps of Engineers.

5.3 SPILLWAY CAPACITY

The spillway has a capacity of 31,345 cfs at the top of the dam. An inflow of 17,706 cfs generated by a storm equal to 1/2 the PMF will produce a maximum outflow of 8901 cfs. An inflow of 35,412 cfs resulting from the PMF will produce a maximum outflow of 22,719 cfs which is well below the spillway capacity of 31,345 cfs at top of dam.

5.4 RESERVOIR CAPACITY

The reservoir capacity to normal pool elevation is 26,067 acre-feet. Surcharge storage to top of dam is an additional 21,823 acre-feet, creating a total storage of 47,890 acre-feet. The surcharge storage between spillway and dam crests is equivalent to 10.5 inches of runoff.

5.5 FLOODS OF RECORD

The maximum known flood of record in Ninemile Creek occurred on June 23, 1972 at a point 1.8 miles downstream from Otisco Lake Dam. Ratioed by drainage area the estimated inflow at Otisco Lake Dam was 913 cfs. The resulting depth of flow over the spillway crest would be about 2.2 feet.

5.6 OVERTOPPING POTENTIAL

The PMF analysis indicates that the dam will not be overtopped by a storm equal to 1/2 the PMF nor by the PMF.

5.7 EVALUATION

The spillway is adequate to pass the flows produced by 1/2 the PMF as well as the PMF without overtopping of the dam. The structure is, therefore, assessed as "Adequate".

SECTION 6: STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

Visual Observation

Both the earth embankment and the masonry/concrete spillway section appeared stable. No sloughing or subsidence was found on the dam.

Design and Construction Data

No information regarding structural stability of the dam or spillway section was located.

c. Operating Records
Any data or information on operations of the Otisco Lake dam can be found at the pumping station located at the dam.

d. Post Construction Changes

The dam as it exists is a reconstruction of a masonry dam that was built in 1907. The overflow spillway was raised and the embankments were paved with concrete.

6.2 STRUCTURAL STABILITY ANALYSIS

A structural stability analysis was conducted for the masonry/concrete gravity spillway portion of the dam. The results of the analysis are as follows:

<u>Case</u>	Overturning Safety Factor	Resultant in Middle Third	Sliding Safety Factor
 a. Normal conditions; water surface at spillway crest 	1.73	Yes	1.03
b. Case a. plus ice load of 5,000 lb/ft	1.30	No	0.76
c. 1/2 PMF flows; water surface 6.4 ft above spillway crest	1.33	No	0.64
d. PMF flows; water surface 9.6 feet above spillway cres	st 1.19	No	0.54
e. Seismic loading; water surface at spillway crest	1.66	Yes	0.73

The analysis indicates that this portion of the dam is marginally stable under normal loading conditions and would be unstable under severe loading conditions (ice loading, flood flows). The analysis was based on available information and was done in accordance with Corps of Engineers "Recommended Guidelines", assuming full uplift pressure under the upstream toe decreasing to tailwater pressure under the downstream toe.

Further investigations are required to better assess the stability of the spillway section. Subsurface explorations and concrete cores are required to obtain information about the uplift forces acting on the dam. An accurate cross section of the spillway should be developed for the analysis. A revised stability analysis should then be performed using this data. Based on the results of these analyses, the need for modifications to the structure should be determined.

This structure is located in Seismic Zone 2. A seismic stability analysis was performed assuming a seismic coefficient of 0.1. The results of this analysis (shown on page 8) indicate that the safety factor against sliding fall below 1.0 when seismic considerations are included.

SECTION 7: ASSESSMENT/RECOMMENDATIONS

7.1 ASSESSMENT

a. Safety The Phase I Inspection of Otisco Lake Dam revealed that the spillway is adequate to pass the Probable Maximum Flood according to the Corps of Engineers screening criteria.

The inspection also revealed that the stability of this structure is questionable. Analysis performed indicated that the structure is only marginally stable under normal loading conditions and is unstable when subjected to severe loading conditions (such as flood flows or iew loading).

b. Adequacy of Information
The information which was available for the preparation of this report
presented a fairly complete history of the structure. Final plans of
the most recent modification did not reveal the actual configuration
of the foundation of the spillway. Therefore, some assumptions had
to be made on the section of the spillway. Overall, the information
is considered adequate for Phase I Inspection purposes.

c. Need for Additional Investigations
Further investigations are required to assess the stability of the spillway section. Subsurface explorations and concrete cores are required to obtain information about the uplift forces acting on the dam. An accurate cross section of the spillway should be developed for the analysis. A revised stability analysis should then be performed using this data. Based on the results of these analyses, the need for modifications to the structure should be determined.

The additional stability investigation must be initiated within 6 months from the date of notification. Within 1 year of notification, remedial measures as a result of these investigations must be initiated, with completion of the measures during the following year. In the interim, develop an emergency action plan for the notification of downstream residents and proper governmental authorities in the event of overtopping and provide round-the-clock surveillance of the dam during periods of extreme run-off.

7.2 RECOMMENDED MEASURES

- 1. The results of the stability investigation will determine the appropriate remedial actions for the spillway section.
- 2. Repair the deterioration of the concrete on the downstream edges of the concrete pavement.
- 3. Repair the undermining of the concrete paving.
- 4. Provide a program of periodic inspection and maintenance of the dam and appurtenances. Document this information for future reference.
- 5. An emergency action plan must be developed and maintained during the life of the structure.

APPENDIX A
PHOTOGRAPHS

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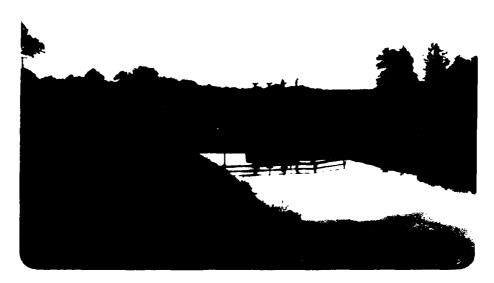


PHOTO #2 DOWNSTREAM VIEW OF DAM



PHOTO # 3 SPILLWAY CREST

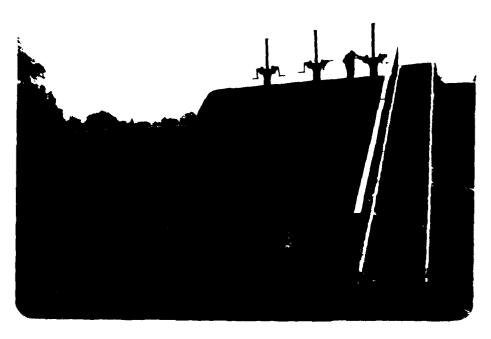


PHOTO # 4 RESERVOIR DRAINS, VALVE CONTROLS



PHOTO # 5. ROADWAY LOCATED BETWEEN SPILLWAY ABUTMENT AND LEFT EARTH EMBANKMENT

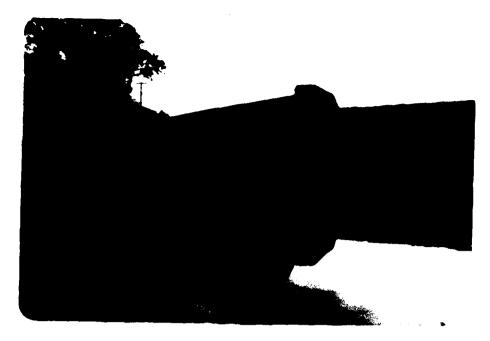


PHOTO # 6 MINOR DETERIORATION OF CONCRETE

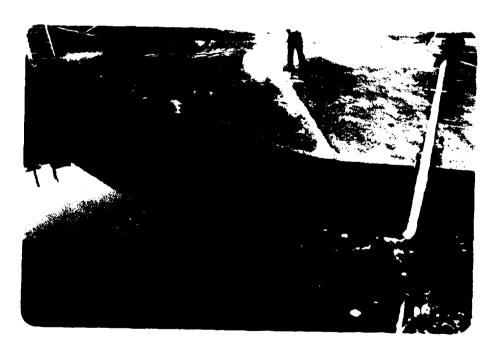


PHOTO # 7 MINOR UNDERMINING OF CONCRETE PAVEMENT



PHOTO # 8 DOWNSTREAM VIEW OF SPILLWAY AND NORMAL TAILWATER

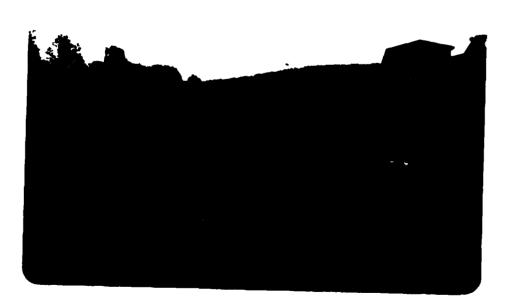


PHOTO # 9 CONCRETE PAVING ON LEFT EMBANKMENT

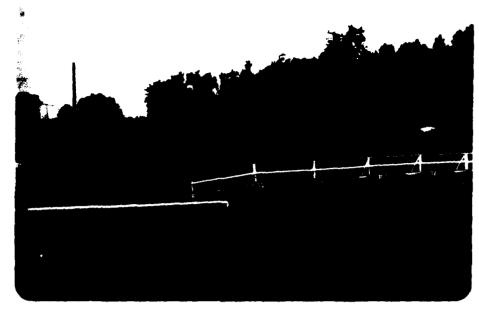


PHOTO # 10 DOWNSTREAM CHANNEL

APPENDIX B
VISUAL INSPECTION CHECKLIST

VISUAL INSPECTION CHECKLIST

1) Basic Data

a.	General
	Name of Dam OTISCO LAKE DAM
	Fed. I.D. # 753 DEC Dam No. 73B-275/
	River Basin OSWEGO RIVER BASIN
	Location: Town MARCELLUS County ONGNOAGA
	Stream Name NINE MILE CREEK
	Tributary of <u>ONONDAGA</u>
	Latitude (N) 42 54.3 Longitude (W) 76 18.8
	Type of Dam EARN W/ Concrete PAVING
	Hazard Category high
	Date(s) of Inspection <u>JUNE 12, 1981</u>
	Weather Conditions //ouny 70 5
	Reservoir Level at Time of Inspection AT PRIMARY SPILLWAY CREST
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~•	Inspection Personnel KEN HARMER, SAME VEITCH
~.	Inspection Personnel KEN HARMER, JAMIE VEITCH
	Persons Contacted (Including Address & Phone No.)
	Persons Contacted (Including Address & Phone No.) Mr. Spery J. De Voldre
	Persons Contacted (Including Address & Phone No.) Mr. Sosepu J. De Voldre Water Raws Manager, Ondnoaga Cty Water Authority
	Persons Contacted (Including Address & Phone No.) Mr. Losean J. De Voldre Water Raws Manager, Ondnorga Cty Water Authority P.O. Box 9, Northern Concurso, Syracuse NY 13211
c.	Persons Contacted (Including Address & Phone No.) Mr. Sosepu J. De Voldre Water Raws Manager, Ondnoaga Cty Water Authority
c.	Persons Contacted (Including Address & Phone No.) Mr. Loseny J. De Voldre WATER RANT MANAGER, ONDNOAGA Cty, WATER AUTHORITY P.O. Box 9, NORTHERN CONCOURSE, SYRACUSE NY 13211 (315) 455-7061
c.	Persons Contacted (Including Address & Phone No.) Mr. Sosen J. De Voldre WATER RANT MANAGER, ONDNOAGA Cty WATER AUTHORITY P.O. Box 9, NORTHERN CONCOURSE, SYRACUSE NY 13211 (315) 455-7061 History:
c.	Persons Contacted (Including Address & Phone No.) Mr. Sepri J. De Voldre White Rowr Manager, Ondnoods Cty Water Authority P.O. Box 9, Northern Concurs, Syracuse NY 13211 (315) 455-7061 History: Date Constructed 1857 Date(s) Reconstructed 1872 1907
c.	Persons Contacted (Including Address & Phone No.) Mr. Sosan J. De Voldre Water Rowr Manager, Ondnoods Cty Water Authority PO. Box 9, Northern Concurs, Syracuse NY 13211 (315) 455-7061 History: Date Constructed 1857 Date(s) Reconstructed 1872 1907

-		
2)	Embankment	

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	(1)	Embankment Material EARTH EMPANEMENTS CONCRETE
		PAVED EARTH ABUTHENITS FORMING AUX. SALLWAY
	(2)	Cutoff Type Concres 75
	(3)	Impervious Core
	(4)	Internal Drainage System pipen TO OUTLET CHANNEL -NO
		FLOW @ TIME OF INSPECTION
	(5)	Miscellaneous
b.	Cres	t
	(1)	Vertical Alignment
	(2)	Horizontal Alignment 4000
	(3)	Surface Cracks None
	(H)	Miscellaneous
	(')	
c.	Upst	ream Slope
	(1)	Slope (Estimate) (V:H) / 3 EARTH PORTION
	(2)	Undesirable Growth or Debris, Animal Burrows NONE
	(3)	Sloughing, Subsidence or Depressions None

(4)	Slope Protection <u>Vegetation</u>
(5)	Surface Cracks or Movement at Toe
Dow	nstream Slope
(1)	Slope (Estimate - V:H) /:3
(2)	Undesirable Growth or Debris, Animal Burrows
(3)	Sloughing, Subsidence or Depressions
(4)	Surface Cracks or Movement at Toe
(5)	Seepage None
(6)	External Drainage System (Ditches, Trenches; Blanket) Not
(7)	Condition Around Outlet Structure excellent
(8)	Seepage Beyond Toe
Abu:	tments - Embankment Contact Good No signs of exosim, Supage

		(1)	Erosion at Contact NoNe-
		(2)	Seepage Along Contact NowE
3)			System ription of System
	b.	Cond	ition of System
	c.	Disc	harge from Drainage System
4)	<u>Ins</u> Pi	trume ezome	ntation (Momumentation/Surveys, Observation Wells, Weirs, ters, Etc.)
			None

3)	Kes	ervoir
	a.	Slopes STABLE
	b.	Sedimentation Not a PROBLEM AROUND DAM
	c.	Unusual Conditions Which Affect Dam
6)	Are	a Downstream of Dam
	a.	Downstream Hazard (No. of Homes, Highways, etc.) Highway rows
		between embankment and abotment
	ъ.	Seepage, Unusual Growth None
	c.	Evidence of Movement Beyond Toe of Dam NONE
	d.	Condition of Downstream Channel
7)	<u>Spi</u>	llway(s) (Including Discharge Conveyance Channel)
	a.	General masony concrete age section
	ъ.	Condition of Service Spillway <u>good Condition</u>

	Condition of Auxiliary Spillway Noncrete Davement - Very
d.	Condition of Discharge Conveyance Channel
Door	numin Dunin (Outlot
Rese	ervoir Drain/Outlet
	Type: Pipe Conduit OtherOther
	Material: Concrete Metal Other Size: 4 ' x 4 ' Length /7
	Size: 4 × 4 Length
	Invert Elevations: Entrance Exit
	Physical Condition (Describe): 7000/ Unobservable
	Material: Concrete
	Joints: <u>could Not SEE</u> Alignment
	Structural Integrity: APPARENTY SOUND
	Hydraulic Capability: Total 750 cfs at Naeman Pool
	Means of Control: Gate Valve Uncontrolled
	Operation: Operable Inoperable Other
	,
	Present Condition (Describe):

	uctural /
a.	Concrete Surfaces
b.	Structural Cracking None
c.	Movement - Horizontal & Vertical Alignment (Settlement)
d.	Junctions with Abutments or Embankments
e.	Drains - Foundation, Joint, Face Not Flowing
f.	Water Passages, Conduits, Sluices good condition
g.	Seepage or Leakage <u>None Fund</u> .

pints - Construction, etc. <u>GOOC</u>
oundation APPARENTLY GOOD - NO SIGNS
SETTLEMENT OF MOVEMENT
butments <u>Good</u>
ontrol Gates OFEIBLE
pproach & Outlet Channels <u>G000</u>
nergy Dissipators (Plunge Pool, etc.) <u>9000 ConDITTon</u>
ntake Structures GBod
tability good
iscellaneous WEU MAINTAINED STRUCTURE

	IMMEDIATELY DOWNSTREAM WELL
	MAINT AINED.
Ope:	ration Procedures (Lake Level Regulation):
	4
	NO SET ROCEDURES, FREPT TO DRAW WATER SUPPLY AND KEEP LAKE LEVEL
	AT CREST

APPENDIX C
HYDROLOGIC/HYDRAULIC
ENGINEERING DATA AND COMPUTATIONS

Otisco Lake

```
Drainage area = 39.15 mi<sup>2</sup>
(Planimetred from anad) = 25,056 acreo

Lake Area (@ El. 788) = 3.71 mi<sup>2</sup>
(Planimetred from anad) = 2374 acreo.

Shoreline = 13.4 mi (Gazetteer)

Max. length of Lake = 5.75 " Feasibility Report for Max. width " = 0.75 " oswego River Naturaled, Max. defeth " = 70 feet May 1978

Normal pool elev. = 786.6" (From Plans as revised)
```

Stage - Capacity

(FABON Fig. A-30, Feasibility Report May 1978)

Elev.	vol. (Acre-Leit)
722	0
780	13,600
785	23 <i>000</i>
788	28,750
790	32,500
795	42,000

otisco Lake

Precipitation:
$$\leq PMP = 21.0"$$
 (H.M. No. 33)

Drainage area = 39.15 mi²

$$L = 12.5 \times \frac{62,500}{12 \times 5280} = 12.33 \text{ mi}.$$

$$L_{ca} = 4.8 \times \frac{62,500}{12 \times 5280} = 4.73$$
 mi.

$$t_p = C_t (L \times Lca)^{0.3}$$

= 2.2(12.33 × 4.73)^{0.3} = 7.45 ht

$$t_{+} = \frac{tp}{5.5} = \frac{7.45}{5.5} = 1.35 \text{ hro}$$

= 1 hr 21 min. Use 1 hr 30 min.

$$Tp = t_p + 0.25(t_{R} - t_{+})$$

= 7.45 + 0.25(1.50 - 1.35)
= 7.45 + 0.04 = 7.49 hx.

El. 7	90.1		El. 786	5.6 D	E	l. 7	90.1	2_	
K	226'	<u></u> ~~í	<i>55</i>	-1		110	· ? 	>	
Total spillway Length = 391' Spillway crest elev. = 786.6 (From Plans as) Assume C = 3.8 (Oger Section) C = 3.4 (Dam)									
EL	<u>L</u> ,	C,	$H_{\mathbf{i}}$	L2	C ₂	H ₂	· а,	Ga =	2
786.6	55	3.8	0	-	-	-	o o	t	0
788	55	3.8	1.4	-	_	-	347	-	347
789	55	3.8	2.4	<u>-</u>	-	-	777	_	777
790-1	55	3.8	3.5		-	-	1369	I	1369
791	55	3.8	4.4	336	3.4	0.9	1929	975	i.
792	55	3.8	5.4	336	3.4	1.9	2623	2992	5615
793	55	3.8	6.4	336	3.4	2.9	3384	5642	9026
794	55	3.8	7.4	336	3.4	3.9	4207	8799	13006
795	55	3.8	8.4	336	3.4	4.9	5088	12391	17479

CHECK LIST FOR DAMS HYDROLOGIC AND HYDRAULIC ENGINEERING DATA

AREA	-CAPA	CITY	DATA:

		Elevation (ft.)	Surface Area (acres)	Storage Capacity (acre-ft.)
1)	Top of Dam	798.1	2973	47890
2)	Design High Water (Max. Design Pool)			***
3)	Auxiliary Spillway Crest	790.1		
4)	Pool Level with Flashboards	***		
5)	Service Spillway Crest	786.6	2291	26067

DISCHARGES

		Volume (cfs)
1)	Average Daily	
2)	Spillway @ Maximum High Water	31345
3)	Spillway @ Design High Water	
4)	Spillway @ Auxiliary Spillway Crest Elevation	
5)	Low Level Outlet	750
6)	Total (of all facilities) @ Maximum High Water	32096
7)	Maximum Known Flood	913
8)	At Time of Inspection	

CREST:	EL	EVATION:
Type: Mason	ry, broad crested	<u> </u>
	Length:	
Location		
SPILLWAY:		
SERVICE		AUXILIARY
786.6	Elevation	None
Ogee	Type	
_	Width	_
	Type of Control	
	Uncontrolled	
	Controlled:	
	Туре	
	(Flashboards; gate)	
	Number	
	Size/Length	
	Invert Material	
	Anticipated Length of operating service	
	Chute Length	
	Height Between Spillway Cres & Approach Channel Invert (Weir Flow)	st

HYDRO	METEROL	OGICAL	GAGES .
HILDRY	me i enve	JAJICAL	WAGES.

Type: Water-Stage recorder # 042401	80
Location: Ninemile Creek. 1.8 mi down stream Otisco Lake Dam. Records:	. from
Date - June 23, 1972	
Max. Reading - 1030 cfs (gage height-8	.65ft.)
FLOOD WATER CONTROL SYSTEM: Warning System:	
Method of Controlled Releases (mechanisms):	

PRAINAGE AREA: 39.15 Soy. Wi	
	_
DRAINAGE BASIN RUNOFF CHARACTERISTICS:	
Land Use - Type: Woods. Open fields Substantial residential d	evo
Terrain - Relief: Moderate to steep slopes, generally well drain	ied.
Terrain - Relief: Moderate to steep slopes, generally well drain High to medium-line, mountain well drain surface - Soil: soils of glacial till origin.	.uca -
Runoff Potential (existing or planned extensive alterations to existing (surface or subsurface conditions)	
No alterations planned or anticipated	
Potential Sedimentation problem areas (natural or man-made; present or future)	ıre)
Potential Backwater problem areas for levels at maximum storage capacity including surcharge storage:	
Significant number of homes and cottages	J
along and/or near the lake shore.	
Dikes - Floodwalls (overflow & non-overflow) - Low reaches along the Reservoir perimeter:	
Location:	
Elevation:	
Reservoir:	
Length @ Maximum Pool	
Length of Shoreline (@ Spillway Crest) 13.4 (Miles)	

NEW YORK STATE DEPT OF ENVIRONMENTAL CONSERVATION FLOOD PROTECTION BUREAU	, 是我也是我也是我们的我们的我们的我们的我们就是我们的我们的我们的,
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PREVIEW OF SEQUENCE OF STREAM NETWORK CALCULATIONS RUNOFF HYDROGRAPH AT ROUTE HYDROGRAPH TO TO SERVE OF NETWORK

DEPT OF ENVIRONMENTAL CONSERVATION **************** **************** COMP 1055 FLOOD PROTECTION BUREAU ***** IAUTO 1786......1427.1140...... UNIT HYBROGRAPH 27 END-OF-PERIOD ORDINATES, LAGE 7.45 HOURS, CP= 0.62 VOL= 1.00 RTIMP EX C S 0 LOCAL NEW YORK STATE NSTAN ISTAGE 0 ALSMX APPROXIMATE CLARK COEFFICIENTS FROM GIVEN SNYDER CP AND TP ARE TC# 5.79 AND R# 4.47 INTERVALS RAIN IPRT 896 CMSTL / INAME MO.DA HR. WN PERIOD ******* HONSI 0----R72 IPLT 0 JPRT 189. STRTL 1.30 MULTI-FLAN ANALYSES TO BE PERFORMED RÀ TI O R12 R24 R48 108.00 118.00 126.00 NPLAN= 1 NRTIO= 6 LRTIO= 1 ME TR C TRACE SUB-AREA RUNOFF COMPUTATION JPLI 1.00 25. RTIOK UNII HYDRGGRAPH DATA 2085. 236. END-OF-PERIOD FLOW JOB SPECIFICATION RECESSION DATA HYDROGRAPH DATA TRSPC • 0.80 CP=0.63 Z ********* LROPT PRECIP DATA ITAPE LOSS DATA 296. 0.00 TRSDA 39.15 COMP I E C ON ERAIN 67-2 0.50 SNAP 0. 86 94.00 1769. 370. IDAY LOSS JOPER 1 P = ICOMP 1.33 1 A 9 E A 39.15 TRSPC COMPUTED BY THE PROGRAM IS 0.843 EXCS 1218. INFLOW FROM BASIN 1S TAG 4 64. DL TK R OTISCO LAKE I UHG RAIN FLOOD HYDROGRAPH PACKAGE (HEC-1) JULY 1978 PHASE 1 MODIFIED FOR HONEYWELL APR 79 26 FEB 79 R T I 05= EXP UNDERFLO AT LOCATION EXP UNDERFLO AT LOCATION 623. 581. STRKR HR.MH PERIOD IHYDG ******** LROPT LAST MODIFICATION DAM SAFETY VERSION RUN DATE 07/21/81 F10.04

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MARNING *** TOP OF DAM, BOTTOM OF B9EACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00
STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

COGD EXPD DAYMID 3.8 1.5 50.

STATION 1, PLAN 1, RATIO 1

END-OF-PERIOD HYDROGRAPH OKDINATES

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307.	268.	373.	1807.	1253.	956.	715.	249.	417.	329.	281.	239.	204.	174.	148.	126.	107.	91.	78.	499
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WARNING *** TOP OF DAM, BOTTOM OF GREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS.IN STORAGE-ELEVATION DATA...
BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00
STOWAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

1. PLAN 1. RATIO 2 STATION

END-OF-PERIOD HYDROGRAPH ORDINATES,

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WARNING *** TOP OF DAM, BOTTOW OF BREACH, OR LOW-LEVEL DUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS.IN STORAGE-ELEVATION DATA Bottom of reservoir assumed to be at 722.00 Storage-elevation data will be extrapolated above elevation 795.00

1. PLAN 1. RATIO 3 STATION

END-OF-PERIOD HYDROGRAPH ORDINATES .

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8901. AT TIME 52.50 HOURS PEAK OUTFLOW IS

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24-HOUR	6164.	175.	5.86	148.80	12226.	15080.
6-HOUR	8619.	244.	2.05	52.02	4274.	5272.
PEAK	8901.	255.				
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MARNING *** TOP OF DAK, BOTTOM OF BREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE..OF GIVEN ELEVATIONS IN...STORAGE-ELEVATION DATA BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00 STORAGE-ELEVATION DATA JILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

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WARNING *** TOP OF DAM, BOTTOM UF RREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA BOTTOM OF RESERVOIR ASSUMED TO BE AT 722.00
STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 795.00

STATION 1, PLAN 1, RATIO 5

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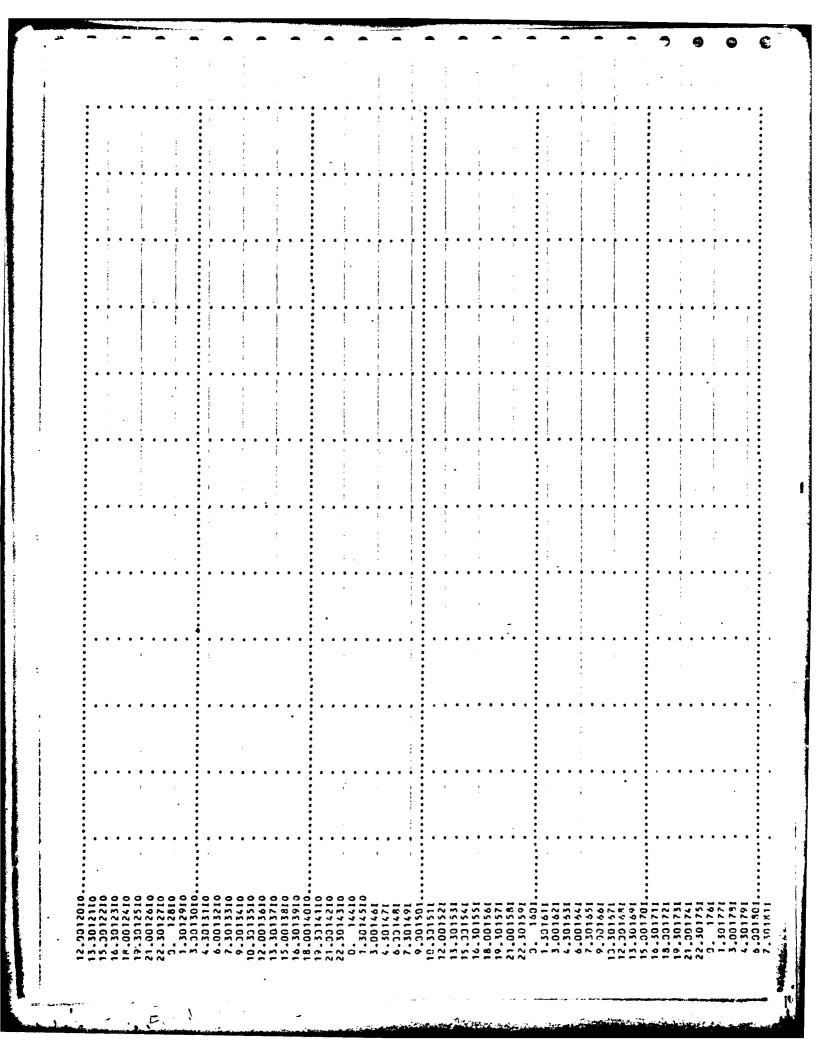
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PEAK OUTFLOW IS . 17322. AT TIME \$1.00 HOURS

TOTAL VOLUME	288373.	8166.	17.13	435.10	35749.	44095.
72-H 0UR	2047.	143.	14.39	365.50	30031.	37042.
24-HOUR	11213.	318.	10.66	270.69	22241.	27434.
6-HOUR	16594.	4.70	3.94	100.15	8228	10149.
PEAK	17322.	491.				
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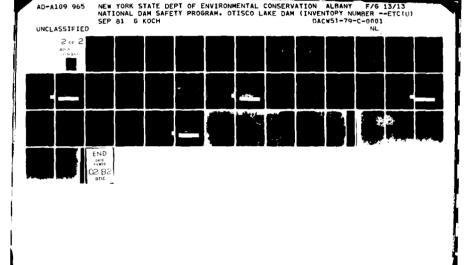
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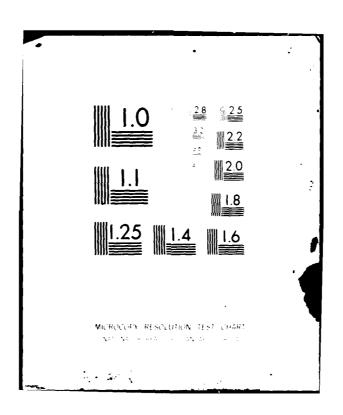
SUMMARY OF DAM SAFETY ANALYSIS

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J 0	RESERVOIR	DEPTH	STORAGE	OUTFL 04	OVER TOP	MAX OUTFLOW	FAILURE	
PMF	W.S.ELEV	OVER DAM	AC-FT	CFS	HOURS	HOUR S	HOURS	
0.20	790.36	•	33178.	1807.	•	57.00	•	
0,.0	792.21		36708.	6347.	•	52.50	•	
0.50	792.96	•	38130.	8901.	•	52,50	• 0	:
0.63	793.67	0	39472	11690.	•	51.00	0.	
0.83	194.96	•	41933.	17322.		51.00	•	
1.00	796.17	•	44226.	22719.	0	49.50	0	•

APPENDIX D
STABILITY ANALYSIS

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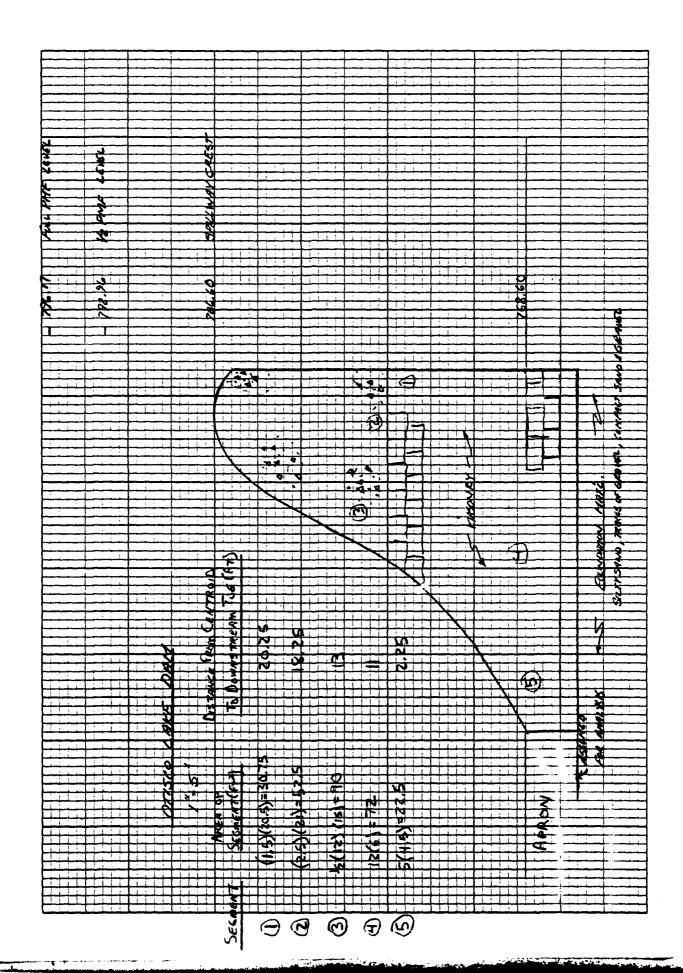


STRUCTURAL STABILITY ANALYSIS

The analysis of the spillway section was based on information shown on the plans and on measurements made at the time of the inspection. A normal analysis was performed including both overturning and sliding analyses. Due to unknown foundation conditions, full uplift was assumed at the upstream toe, decreasing to the tailwater pressure at the downstream toe. It was assumed that the base of the dam is at the same level as the bottom of the spillway apron (which is 3 feet thick).

ANALYSIS CONDITIONS

- 1. Normal conditions; water surface at spillway crest
- 2. Same as #1 plus ice load of 5,000 pounds per linear foot
- 3. 1/2 PMF flow; water surface 6.4 feet above the spillway crest
- 4. PMF flow; water surface 9.6 feet above spillway crest
- 5. Seismic Conditions Water at Spillway Crest with seismic coefficient of 0.1.



STABILITY ANALYSIS PROGRAM - WORK SHEET

INPUT EKTRY				IS CONDI	TION	
Unit Weight of Dam (K/ft ³)	0	0.15	0,15	0,15	0.15	10.15
Area of Segment No. ! (ft ²)	1	30.75	30,75	30.75	30.75	30.75
Distance from Center of Gravity of Segment No. 1 to Downstream Toe (ft)	2	20.25	20.25	20,75	20.25	20,25
Area of Segment No. 2 (ft ²)	3	52,5	52.5	52,5	\$ 2.5	5 2.5
Distance from Center of Gravity of Segment No. 2 to Downstream Toe (ft)	4	18.25	18,25	18.25	18,23	18,25
Area of Segment No. 3 (ft ²)	5	90	90	90	90	90
Distance from Center of Gravity of Segment No. 3 to Downstream Toe (ft)	6	13	13	13	13	13
Base Width of Dam (Total) (ft)	7	21	21	اخ	51	اح
Height of Dam (ft)	8	21	21	51	21	21
Ice Loading (K/L it.)	9		5.0	_	-	
Coefficient of Sliding	10	0.55	0.55	0.55	0.55	0,55
Unit Height of Soil (K/ft3) (defict 18)	11	0,055	0.055	0.055	0,055	0,055
Active Soil Scefficient - Ka	12	0.33	0,33	0.33	0.33	0.33
Passive Soil Coefficient - Kp	13	3.00	3.0	2.0	3.0	3.0
Height of Water over Top of Dam on Opillway (ft)	14		_	6.36	9,57	
Height of Soil for Active Pressure (ft)	15	3.0	3.0	3.0	3,0	3.0
Height of Soil for Passive Pressure (ft)	16	3.0	3,0	3,0	3.0	3.0
Height of Water in Tailrace Channel (ft)	17	-5,0	5.0	7.0	7.0	S. O
Weight of Water (K/ft ³)	18	.0624	.0624	,0624	. 0624	,0624
Area of Segment No. 4 (ft ²)	19	72	72	72	72	72
Distance from Center of Gravity of Segment No. 4 to Downstream Toe (ft)	20	11	u.	u	11	11
Height of Ice Load or Active Water (ft) (does not include 14)	46	51	21	21	15	2/
Seismic Coefficient (g)	50	-	_	-	_	0,1
AREA OF SEG. 5	15	22.5	22,5	22.5	22.5	22,5
RESULTS OF AMPLYSIS DIST, TO CENTROLO TO 0,5, Toe Sec. 5	22	2.25	2,25	2,25	2,25	7.25,
Factor of Safety vs. Overturning _		1.73	1'30	1.33	1,19	1.66
Distance From Toe to Resultant		991	5,37	6.12	4.15	9,33
Factor of Safety vs. Sliding		1,03	0,76	0.64	0,54	0.73

APPENOIX E

REFERENCES

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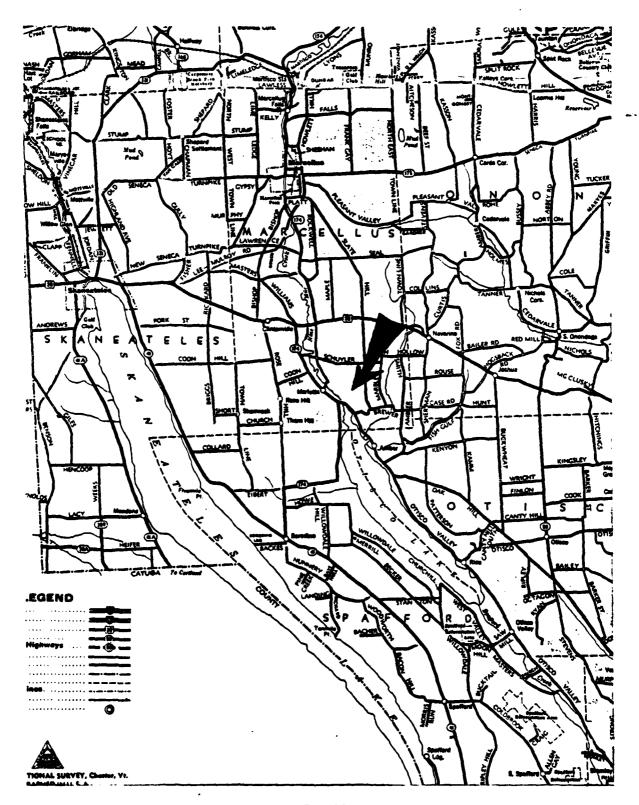
APPENDIX E

REFERENCES

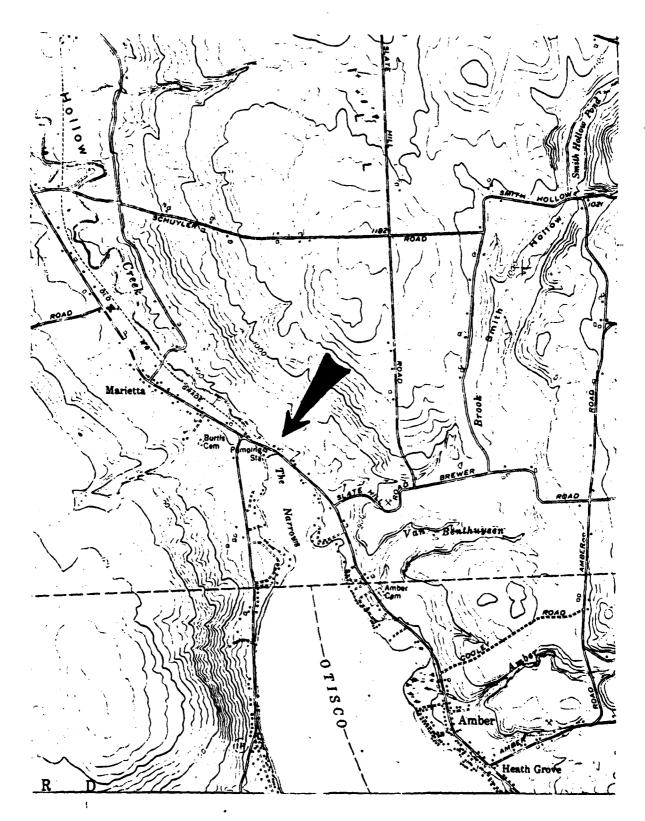
- 1) U.S. Department of Commerce, Technical Paper No. 40, Rainfall Frequency Atlas of the United States, May 1961,
- 2) U.S. Department of Commerce, Hydrometeorological Report No. 33, Seasonal Variation of the Probable Maximum Precipitation East of the 105th Meridian for Areas from 10 to 1,000 Square Miles and Curations of 6, 12, 24, and 48 Hours; April 1956.
- 3) Soil Conservation Service, <u>National Engineering Handbook</u>, Section 4, Hydrology, August 1972 (U.S. Department of Agriculture),
- 4) H.W. King and E.F. Brater, <u>Handbook of Hydraulics</u>, 5th edition, McGraw-Hill, 1963.
- 5) T.W. Lambe and R.V. Whitman, <u>Soil Mechanics</u>, John Wiley and Sons, 1965.
- 6) W.D. Thornbury, <u>Principles of Geomorphology</u>, John Wiley and Sons, 1969.
- 7) University of the State of New York, <u>Geology of New York</u>, Education Leaflet 20, Reprinted 1973.
- 8) Cornell University Agriculture Experiment Station (compiled by M.G. Cline and R.L. Marshall), General Soil Map of New York State and Soils of New York Landscapes, Information Bulletin 119, 1977,

APPENDIX E

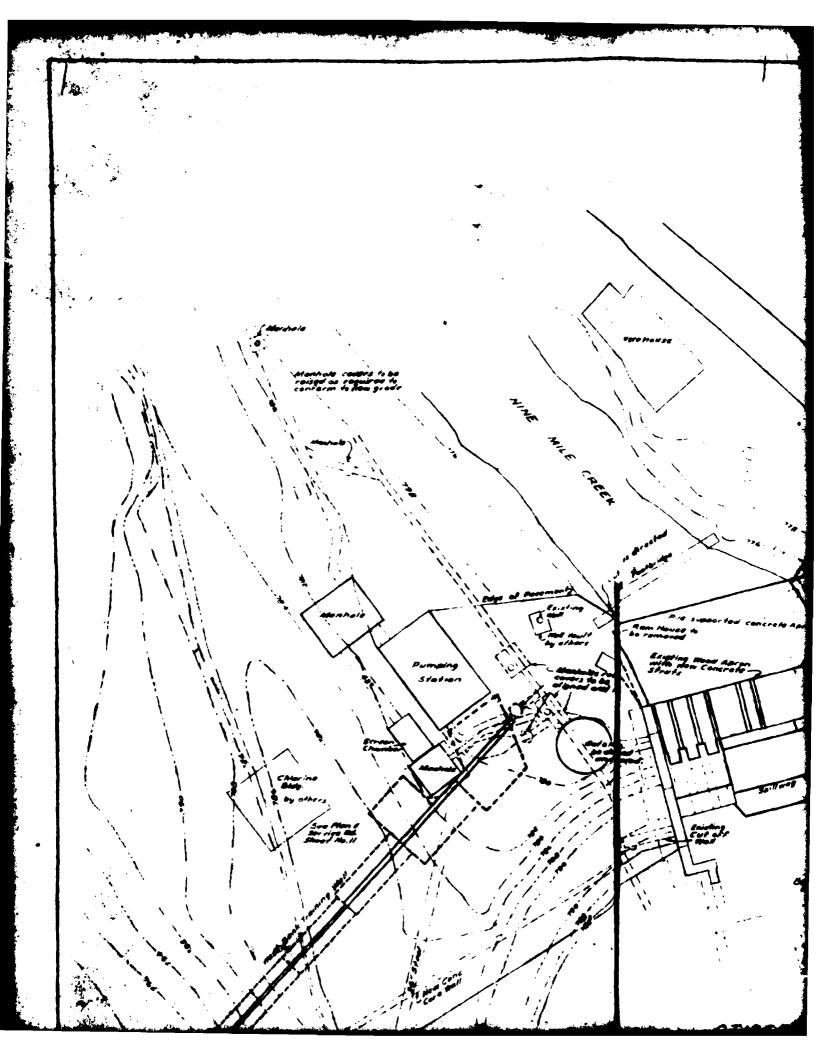
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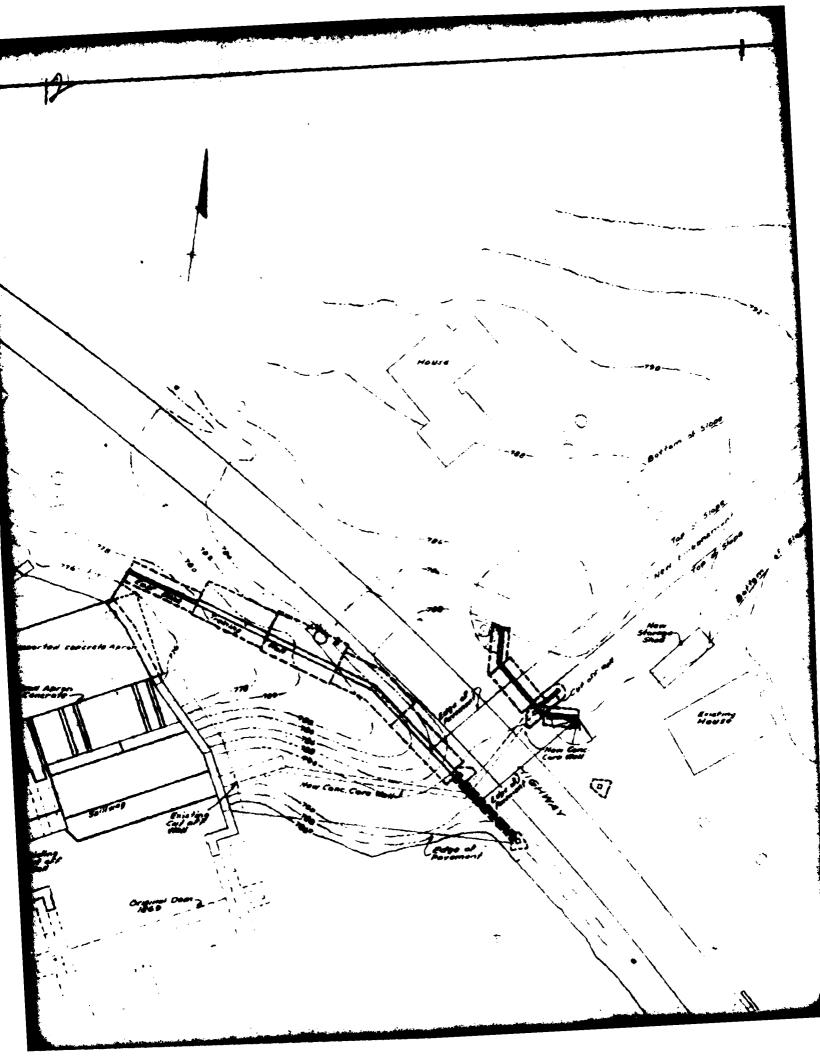


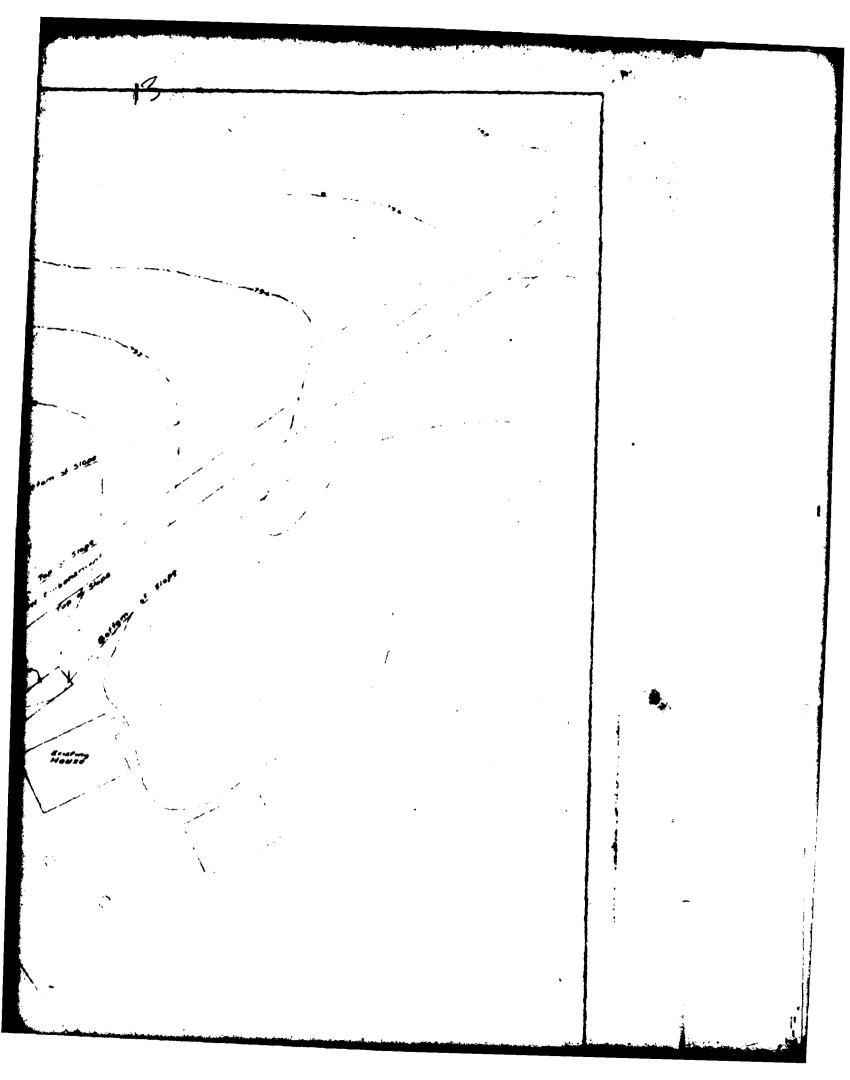
VICINITY MAP

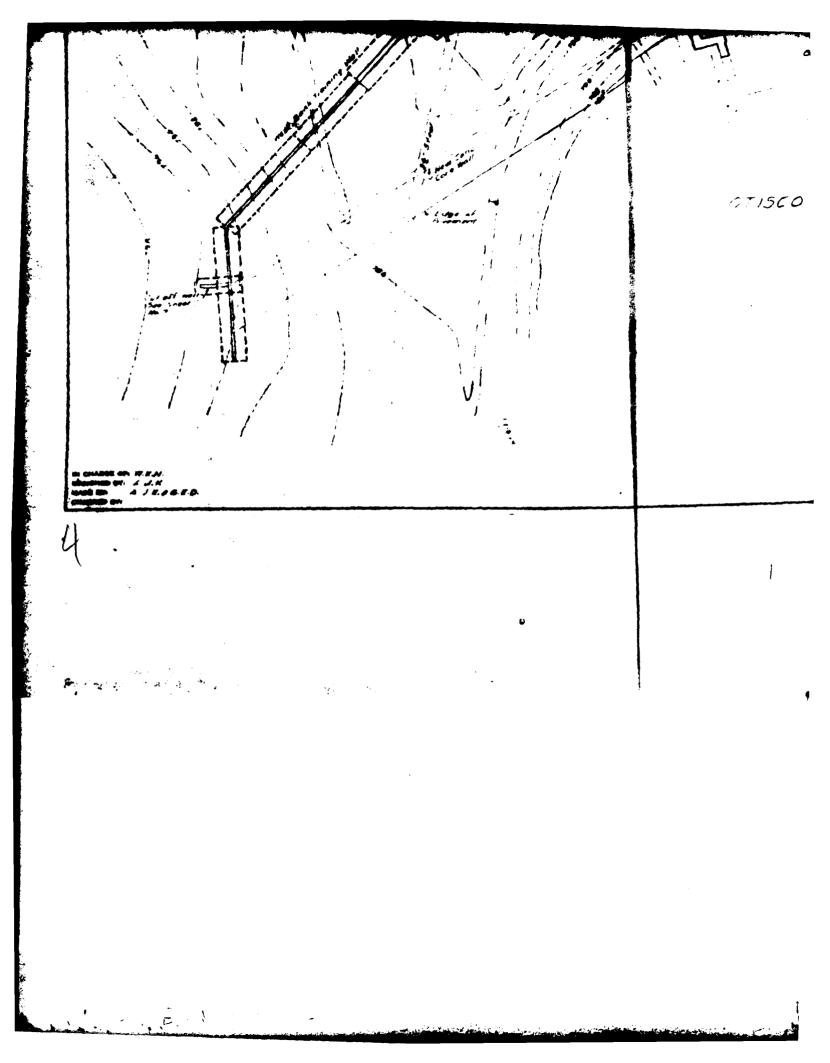


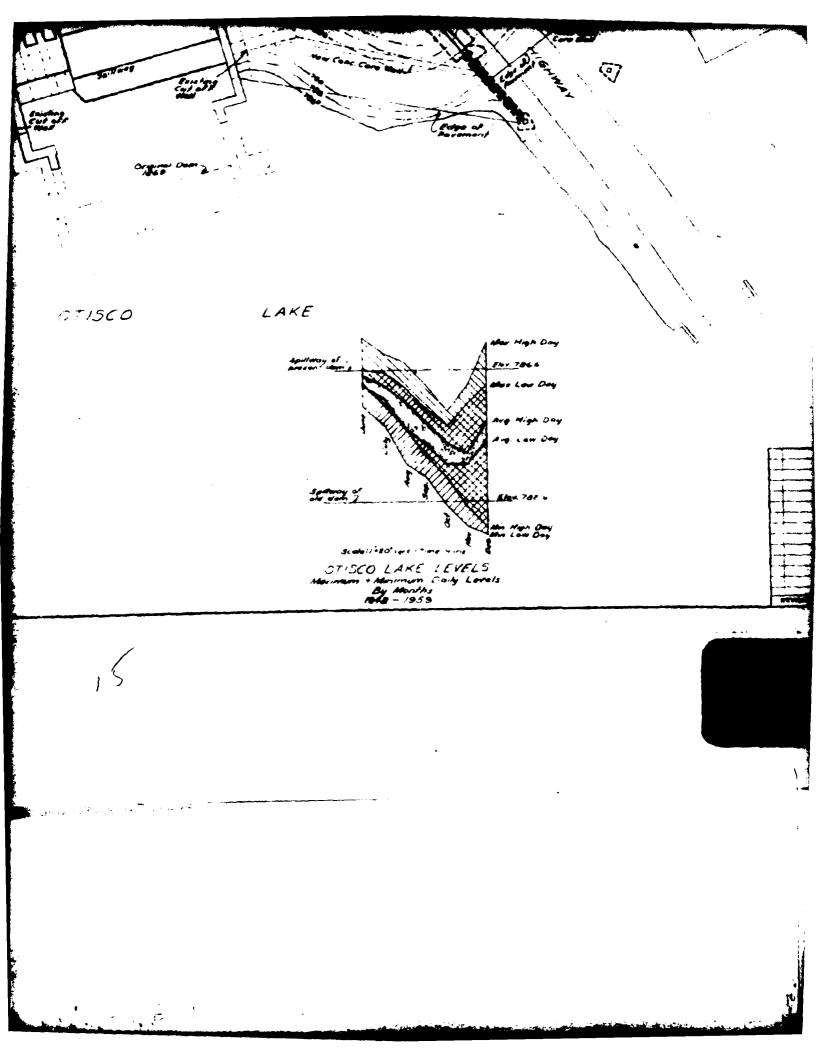
TOPOGRAPHIC MAP









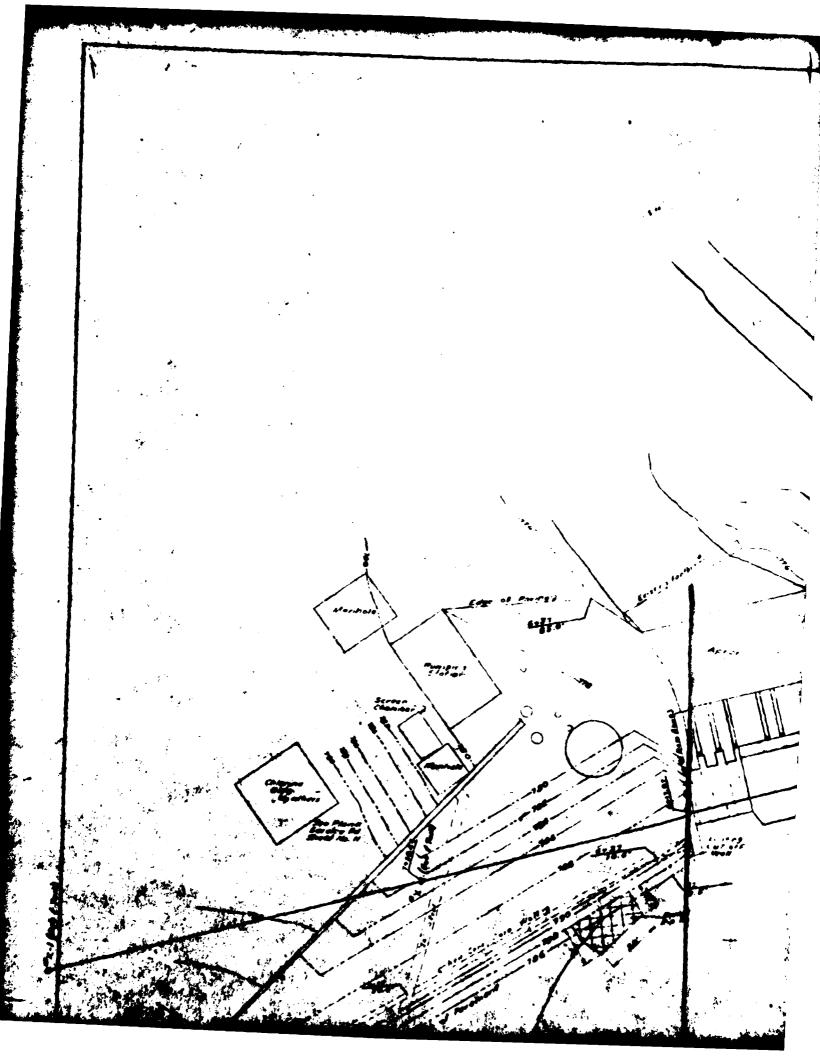


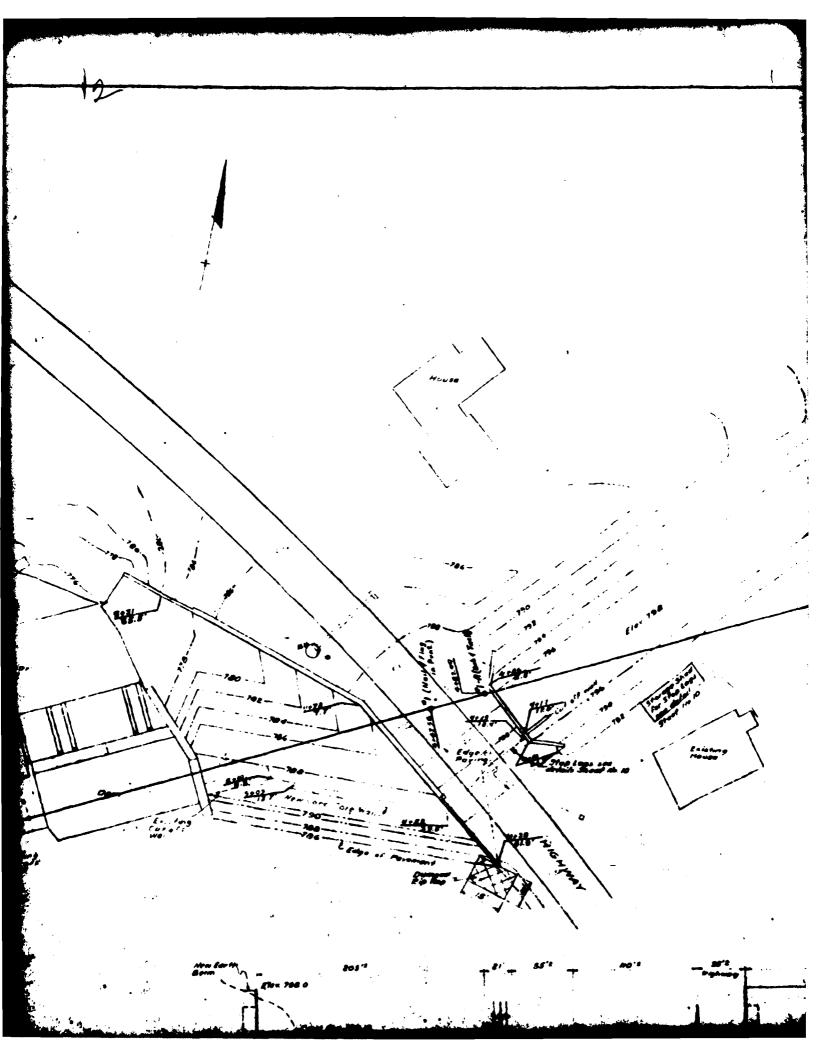
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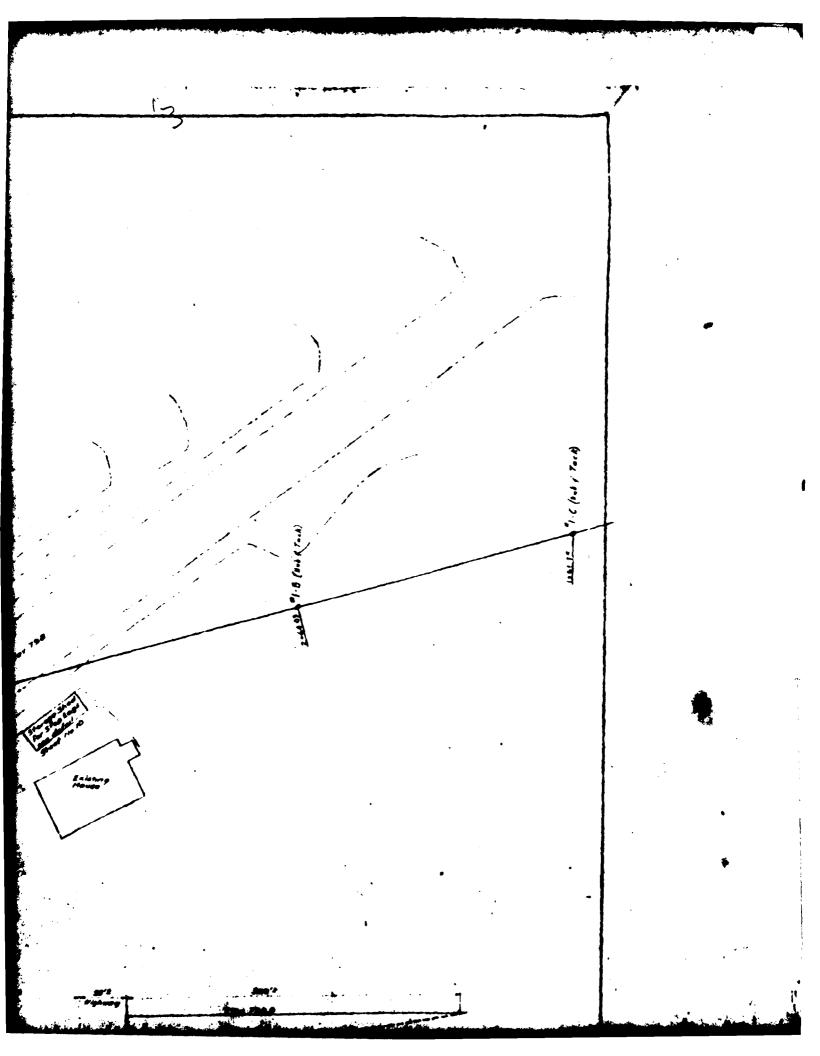
ONONDAGA COUNTY WATER AUTHORITY	
OTISCO LAKE DAM MODIFICATIONS	
GENERAL PLAN	
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	OTISCO LAKE DAM MODIFICATIONS GENERAL PLAN

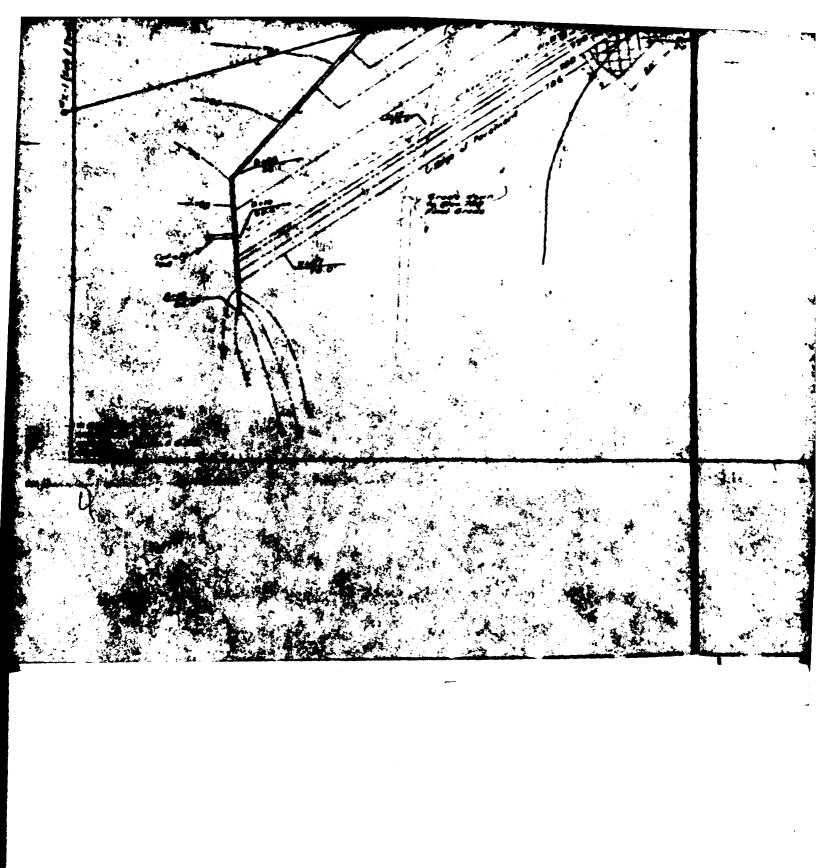
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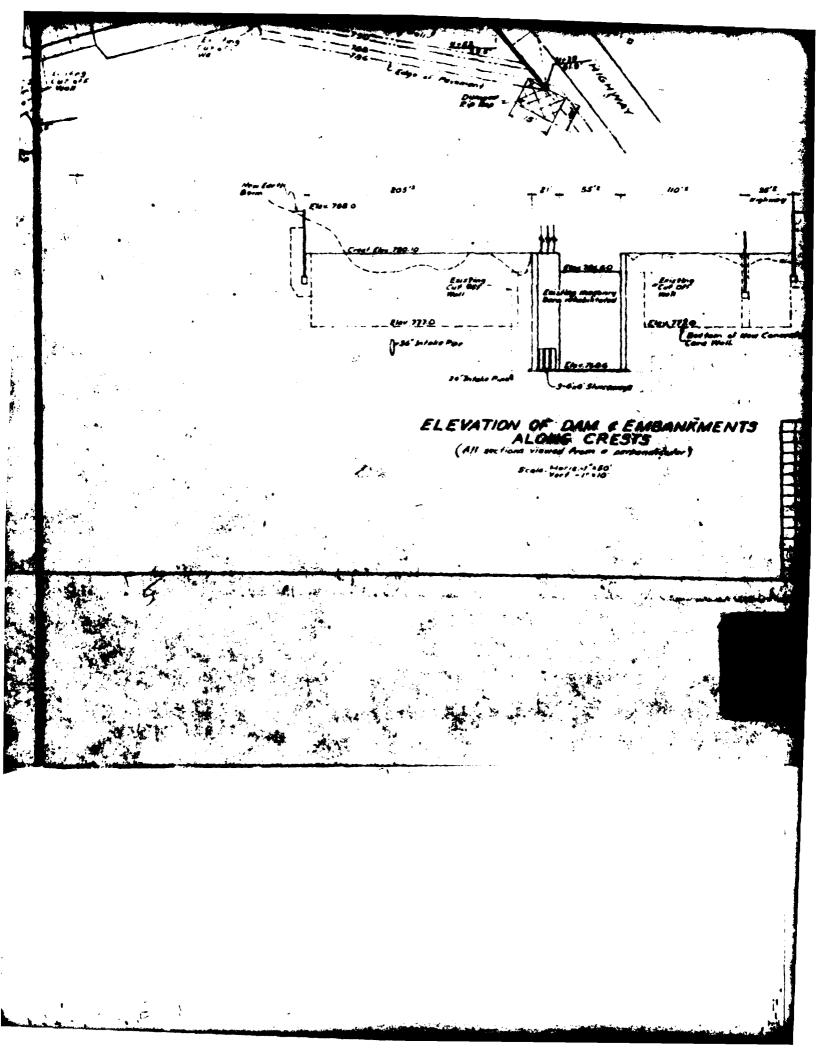


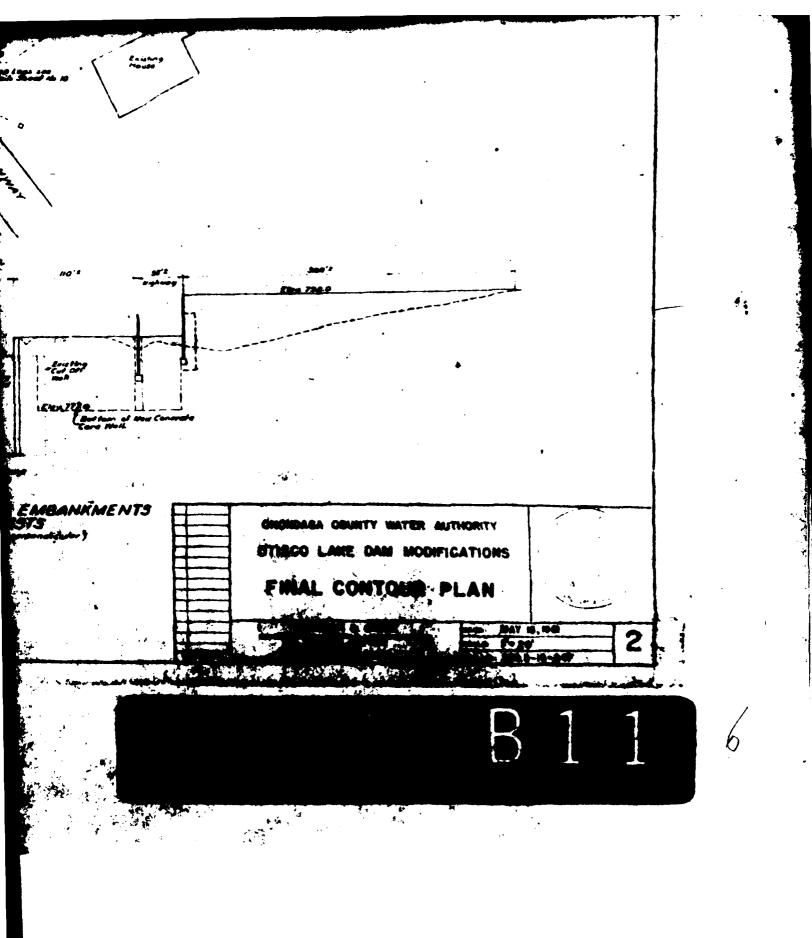






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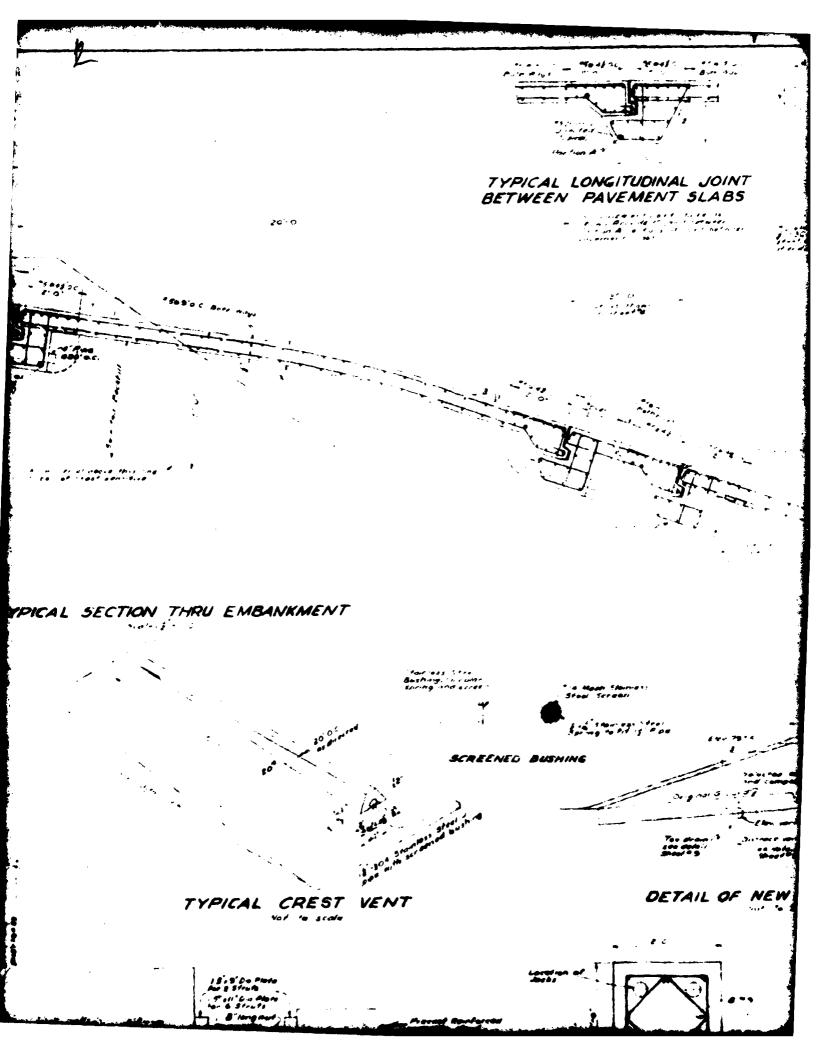




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TYPICAL TRANSVERSE JOINT BETWEEN PAVEMENT SLABS

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JOINT SLABS

AIR VENT DETAIL

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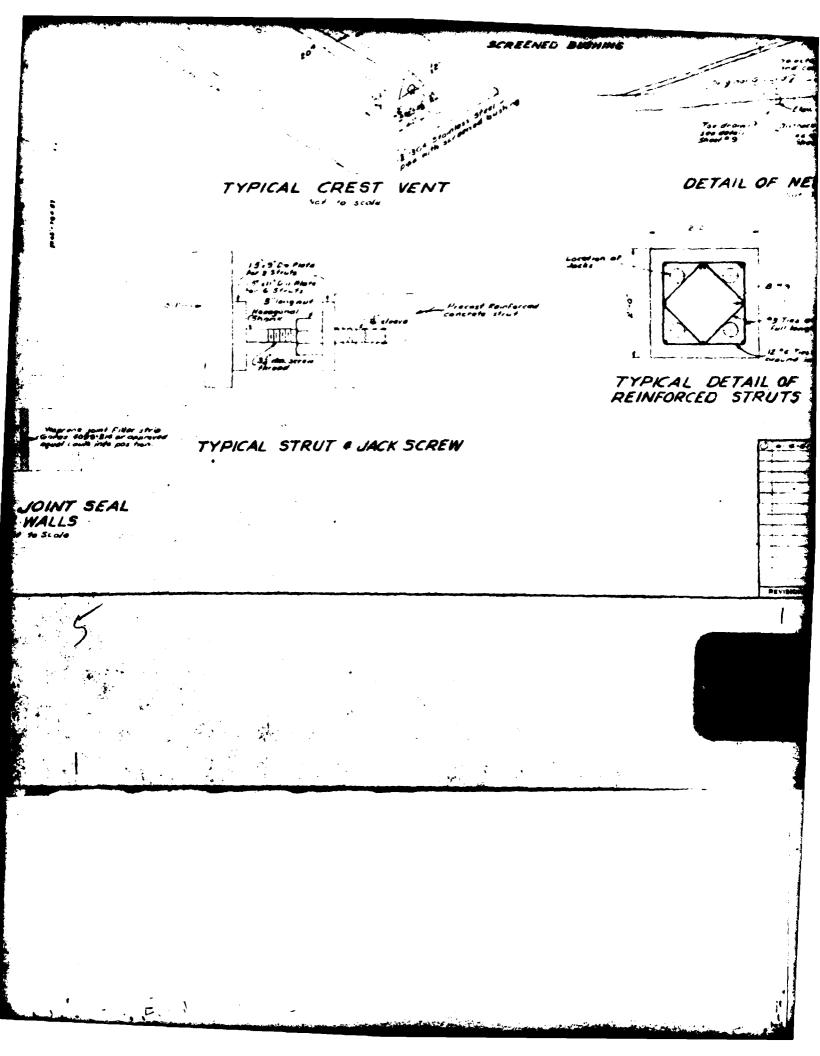
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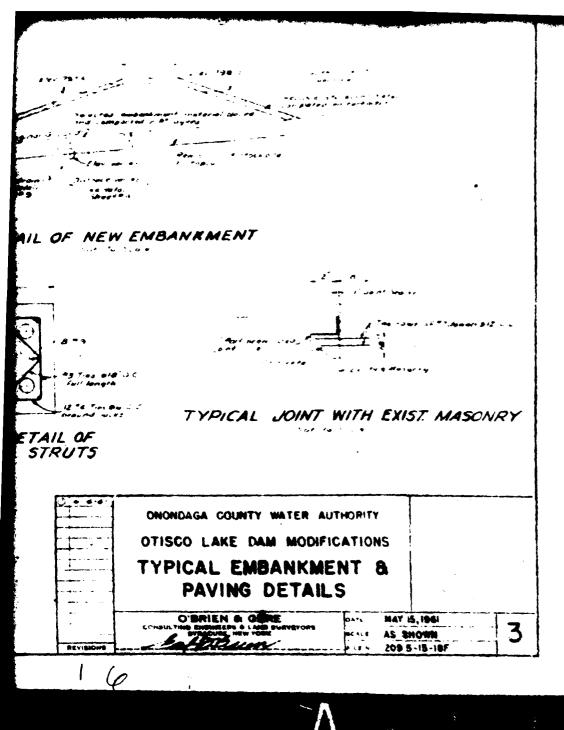
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TYPICAL JOINT SEAL FOR MAVEMENT SLAB

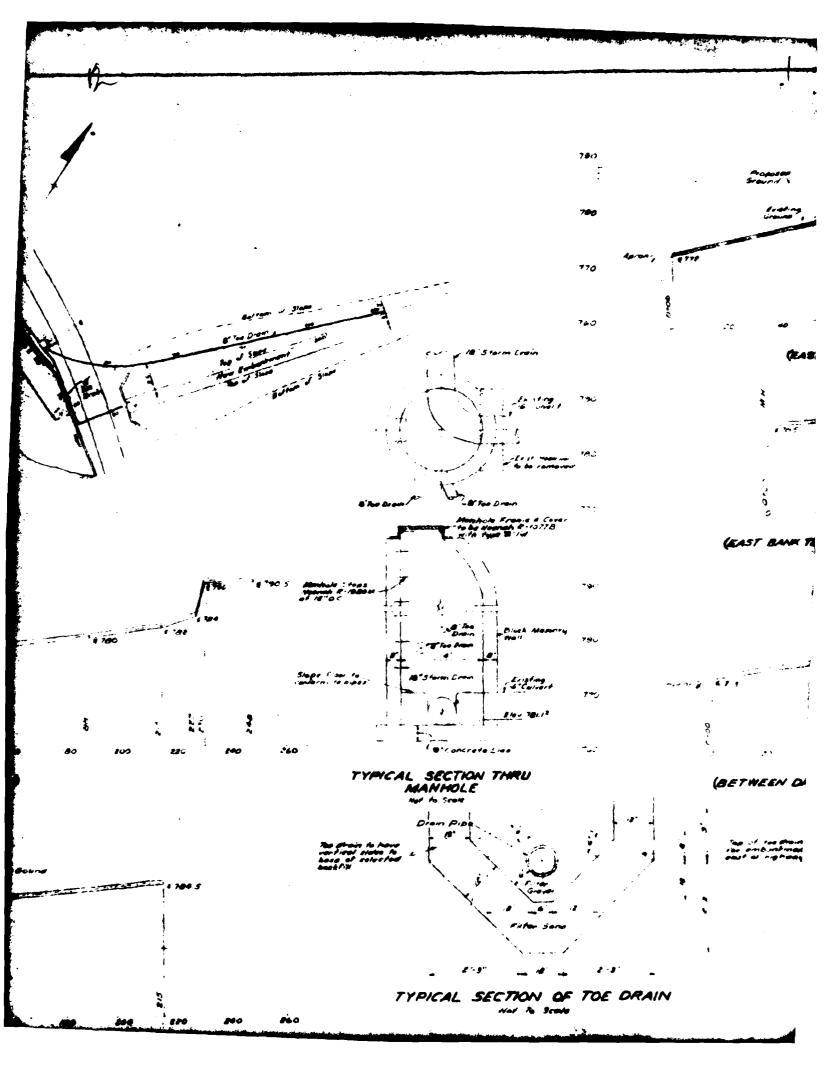
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WALLS





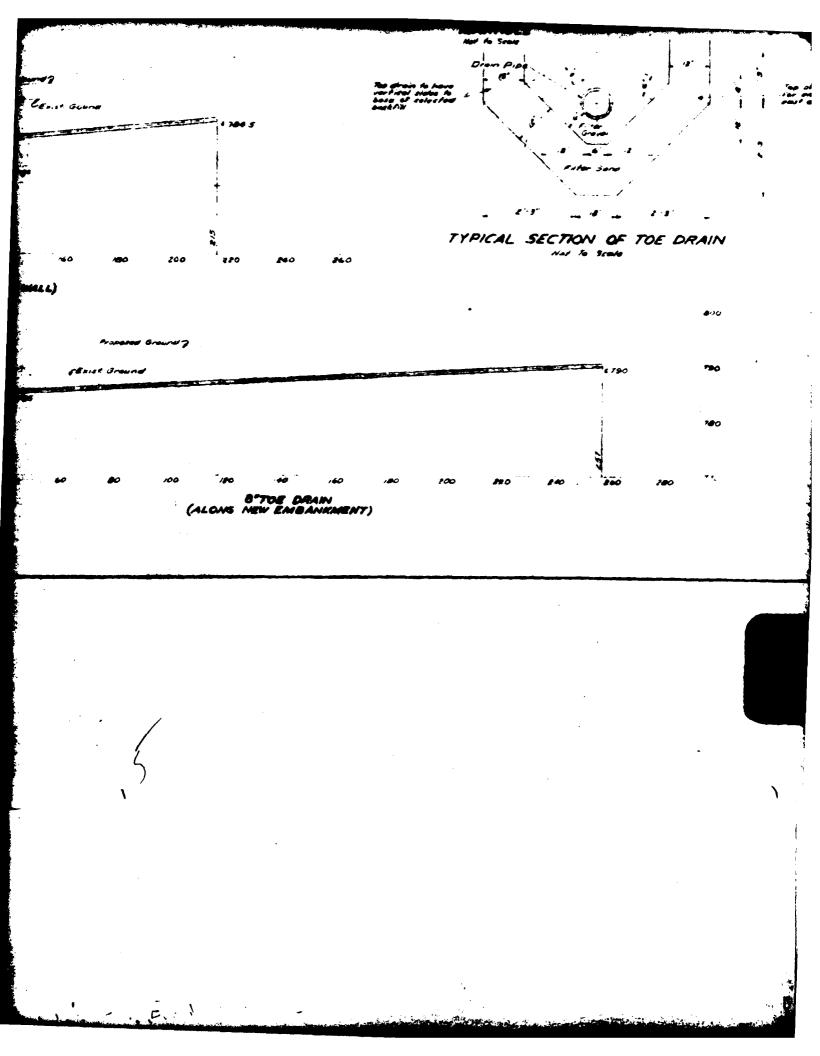
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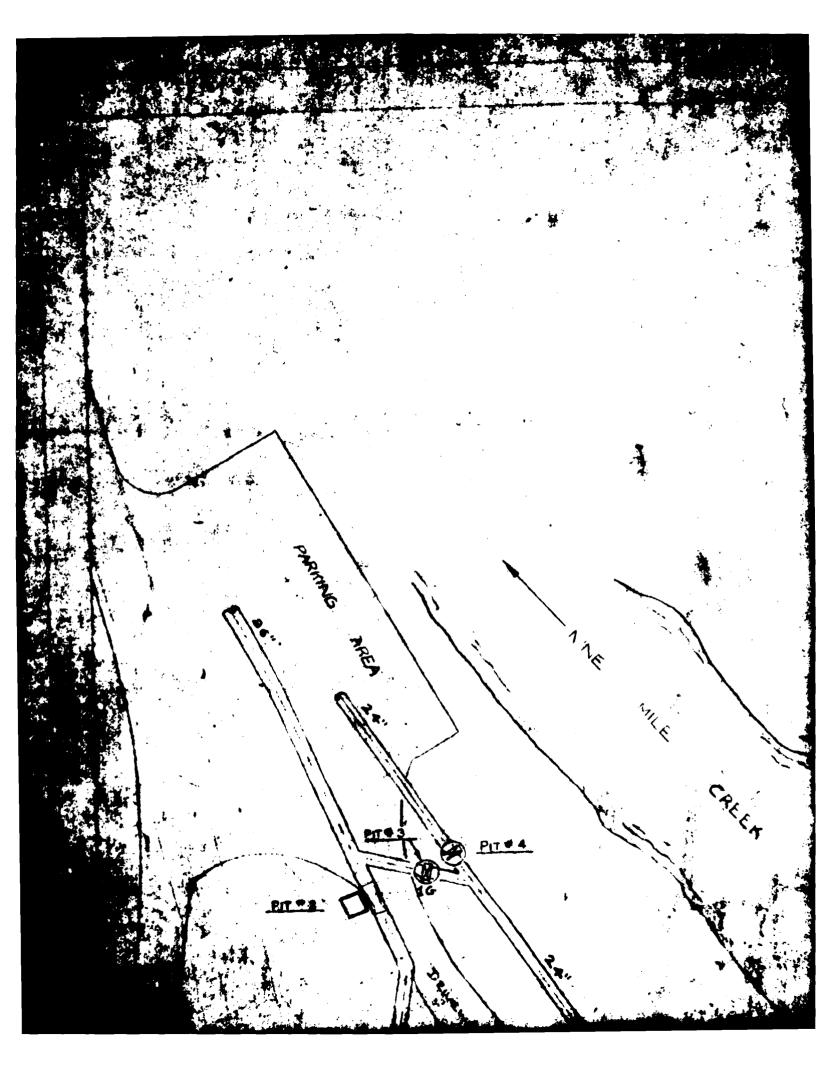
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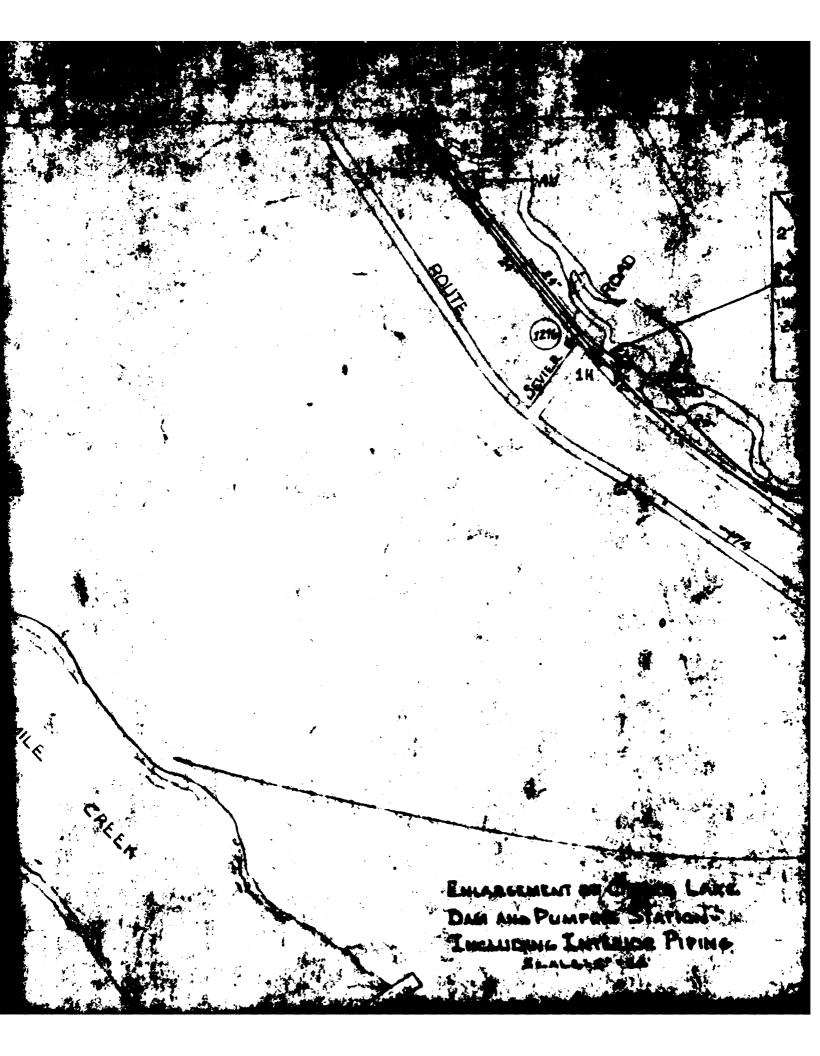
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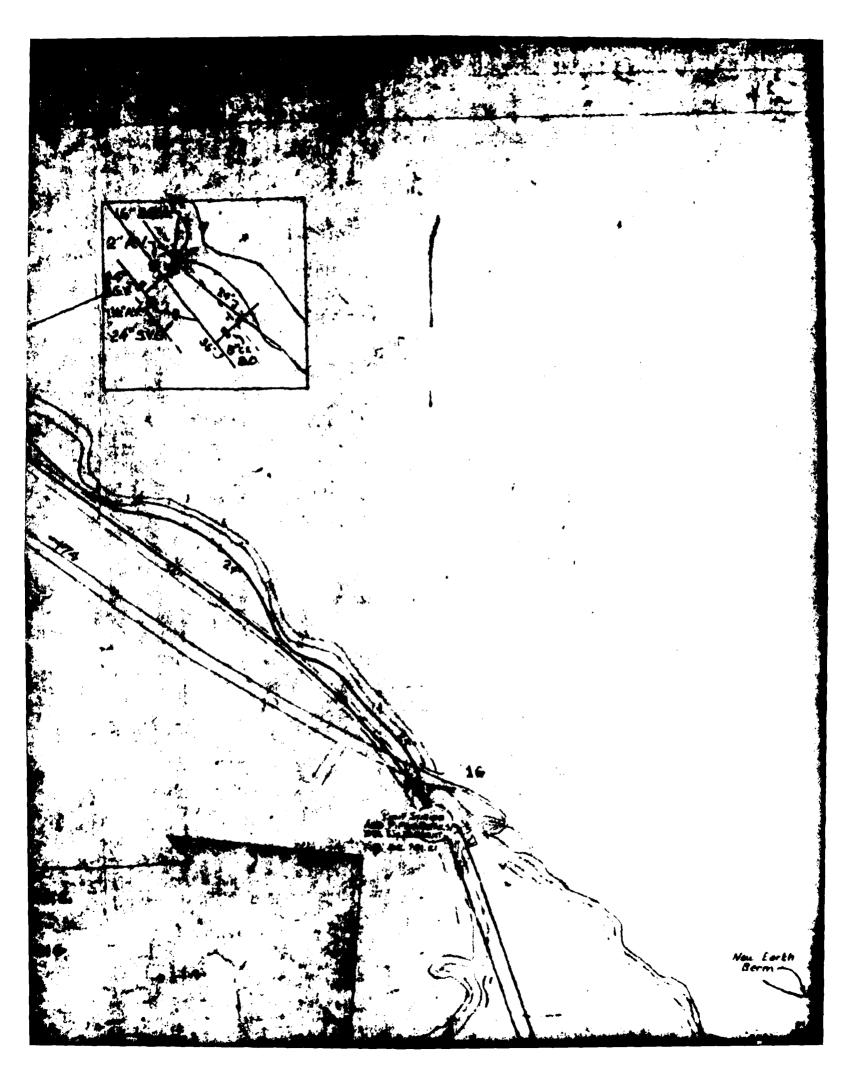
1 779 80.00 760 . B'WALL DAMM BANK TRANSON WALL) ::



(BETWEEN DAM & EAST BANK TRAINING WALL) TYPICAL SECTION OF WALL DRAIN Var to serve ONONDAGA COUNTY WATER AUTHORITY OTISCO LAKE DAM MODIFICATIONS **DRAINS**







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			35 Dise
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			30, CM
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4	No	VALVE PH	54, D4
5	YES	WELL PIE	AUNILIA
			TENAN
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7	No	VALVE PIT	24" BY
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100	30" CHECK VALVE	
MUSMITTER DIT	The state of the s	31
	FLOW METER TRANSMITTER	- 33
ALVE PIT	24" DISCHARGE VALVE, MINRIAL GARAGES	
ALVE PIT	24" DISCHARGE VALVE, MANUAL GATE-OFM	
VELL PITA	AUXILIARY CHLORINATOR SUPPLY WELL PLANE	7
	TENANT HOUSE WATER PUMPS THANK	1. A. I.
ENTURI PIT	24" = 16" VENTURI, 12" SAMPLE PUMP	
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ALVE PIT	24" BY-PASS VALVE, MAN. GATE-CLOSED.	
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D VALVE HOUSE PIT	5-24" SUCTION VALVES, MANUAL GATES	1 1 1 1
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